GENERAL NOTES

1. ALL STRUCTURAL ITEMS FOR THIS PROJECT HAVE BEEN DESIGNED IN ACCORDANCE WITH APPROPRIATE PROVISIONS OF EACH OF THE FOLLOWING:

   A. BUILDING CODE: VIRGINIA UNIFORM STATEWIDE BUILDING CODE(VUSBC) 2018
   B. STRUCTURAL STEEL: THE A.I.S.C. "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS ANSI/AISC 360."
   C. CONCRETE: A.C.I. "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", ACI 318.

2. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS AND ALL OTHER DISCIPLINES DRAWINGS (INCLUDING ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL). IF THERE ARE QUESTIONS BETWEEN DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT AND/OR STRUCTURAL ENGINEER IN WRITING PRIOR TO PERFORMING WORK.
3. IN ANY CASE OF QUESTIONS BETWEEN THE NOTES, DETAILS, AND SPECIFICATIONS, THE MOST RIGID REQUIREMENTS SHALL GOVERN.
4. DETAILS DESIGNATED AS "TYPICAL" APPLY TO ALL AREAS OF SIMILAR CONDITIONS UNLESS OTHERWISE NOTED.
5. MECHANICAL/PLUMBING/ELECTRICAL OPENINGS SHALL BE COORDINATED BY CONTRACTOR. FINAL SIZES AND LOCATIONS TO BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL.
6. CONTRACTOR IS RESPONSIBLE FOR AND SHALL VERIFY AND COORDINATE ALL DIMENSIONS, DETAILS, AND EXISTING CONDITIONS BEFORE PROCEEDING WITH WORK. ANY DISCREPANCIES SHALL BE BROUGHT IN WRITING TO THE IMMEDIATE ATTENTION OF THE ARCHITECT AND/OR STRUCTURAL ENGINEER.
7. CONTRACTOR SHALL FULLY BRACE AND OTHERWISE PROTECT ALL WORK IN PROGRESS UNTIL THE STRUCTURE IS COMPLETED.
8. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR CONCRETE AND STRUCTURAL STEEL A MINIMUM OF TWO (2) WEEKS PRIOR TO THE START OF FABRICATION.
9. THE OWNER SHALL ENGAGE AN INDEPENDENT TESTING AND INSPECTION AGENCY ACCEPTABLE TO THE ARCHITECT AND/OR STRUCTURAL ENGINEER TO INSPECT THE FOLLOWING:

   A. SOIL
   B. STEEL - HIGH STRENGTH BOLTED CONNECTIONS AND WELDED CONNECTIONS IN THE SHOP AND FIELD
   C. CONCRETE - INSPECT REINFORCING PLACEMENT, INSPECT AND TEST CONCRETE QUALITY

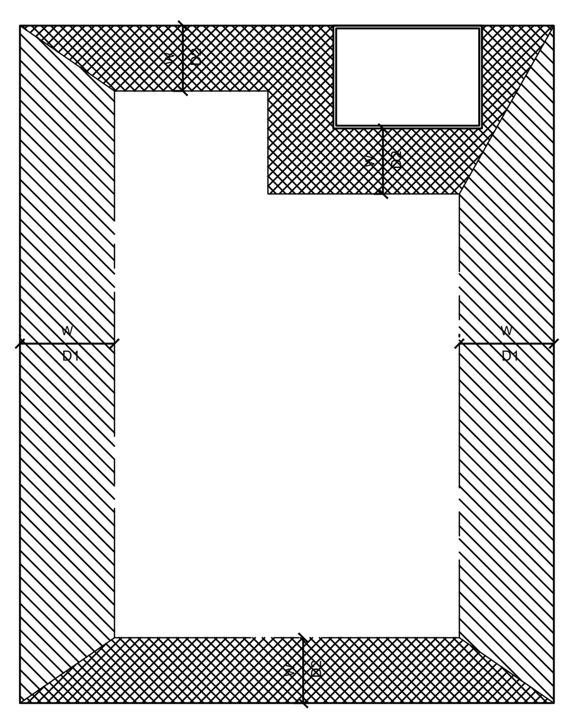
   CONTRACTOR SHALL COORDINATE INSPECTIONS REQUIRED FOR THE ABOVE AGENCIES.
10. ALL REQUESTS FOR SUBSTITUTIONS OF MATERIALS OR DETAILS SHOWN IN THE CONTRACT DOCUMENTS SHALL BE SUBMITTED FOR APPROVAL DURING THE BIDDING PERIOD. ONCE BIDS ARE ACCEPTED, PROPOSED SUBSTITUTIONS WILL BE CONSIDERED ONLY WHEN THEY ARE OFFICIALLY SUBMITTED WITH AN IDENTIFIED SAVINGS TO BE DEDUCTED FROM THE CONTRACT.
11. THE WORK SHALL BE IN ACCORDANCE WITH APPROVED SUBMITTALS EXCEPT THAT THE CONTRACTOR SHALL NOT BE RELIEVED OF RESPONSIBILITY FOR DEVIATIONS FROM THE REQUIREMENTS OF THE CONTRACT DOCUMENTS BY THE ARCHITECT'S APPROVAL OF SHOP DRAWINGS, PRODUCT DATA, SAMPLES, OR SIMILAR SUBMITTALS, UNLESS THE CONTRACTOR HAS SPECIFICALLY NOTIFIED THE ARCHITECT OF SUCH DEVIATION AT THE TIME OF SUBMITTAL AND (1) THE ARCHITECT HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION AS A MINOR CHANGE WORK, OR (2) A CHANGE ORDER OR CONSTRUCTION CHANGE DIRECTIVE HAS BEEN ISSUED AUTHORIZING THE DEVIATION. THE CONTRACTOR SHALL NOT BE RELIEVED OF RESPONSIBILITY FOR ERRORS OR OMISSIONS IN SHOP DRAWINGS, PRODUCT DATA, SAMPLES, OR SIMILAR SUBMITTALS, BY THE ARCHITECT'S APPROVAL THEREOF.

DESIGN CRITERIA

1. SEE PLANS FOR ROOF DEAD AND LIVE LOADS
2. SNOW LOADS:

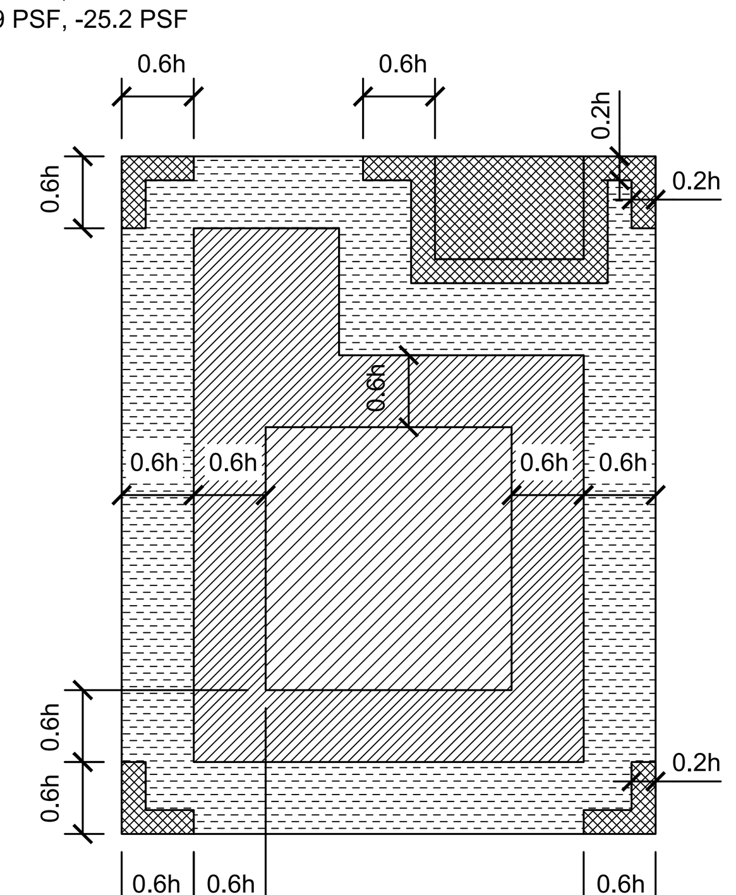
   GROUND SNOW LOAD, Pg = 35 PSF
   FLAT ROOF SNOW LOAD, Pf = 27.0 PSF *
   MINIMUM SNOW LOAD USED FOR DESIGN = 27.0 PSF
   SNOW EXPOSURE FACTOR, Ce = 1.0
   SNOW LOAD IMPORTANCE FACTOR, I = 1.0
   THERMAL FACTOR, Ct = 1.1

   * FLAT ROOF SNOW LOAD TO BE ADJUSTED PER CODE FOR DRIFT, SLIDING, UNBALANCED LOADING, ETC.



3. WIND LOADS:

   BASIC WIND SPEED
   Vult = 115 MPH   Vasd = 89 MPH
   RISK CATEGORY - II
   WIND EXPOSURE - B
   INTERNAL PRESSURE COEFFICIENT GCpi = ±0.18
   COMPONENTS AND CLADDING PRESSURE:
   a = 6.64 FT
   ROOF:
   ZONE 1' P = +7.30 PSF, -18.4 PSF
   ZONE 1 P = +7.30 PSF, -30.3 PSF
   ZONE 2 P = +7.30 PSF, -39.7 PSF
   ZONE 3 P = +7.30 PSF, -52.4 PSF
   WALLS:
   FIELD P = +19.9 PSF, -20.9 PSF
   CORNERS P = +19.9 PSF, -25.2 PSF



4. SEISMIC LOADS:

   SEISMIC RISK CATEGORY - II
   SEISMIC IMPORTANCE FACTOR, I = 1
   MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS:
   Ss = 0.125g
   S1 = 0.044g
   SITE CLASS - C
   DESIGN SPECTRAL RESPONSE COEFFICIENTS:
   Sds = 0.109g
   Sd1 = 0.044g
   SEISMIC DESIGN CATEGORY - A
   BASIC SEISMIC-FORCE-RESISTING SYSTEM:
   LIGHT FRAMED WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE
   DESIGN BASE SHEAR, V = 7.6 KIPS
   SEISMIC RESPONSE COEFFICIENT, Cs = 0.017
   RESPONSE MODIFICATION FACTOR, R = 6.5
   ANALYSIS PROCEDURE - EQUIVALENT LATERAL FORCE PROCEDURE

FOUNDATION CONSTRUCTION NOTES

1. FOUNDATIONS FOR THIS PROJECT CONSIST OF SPREAD AND STRIP FOOTINGS DESIGNED TO BEAR ON APPROVED RESIDUAL MATERIALS OR NEW UNDISTURBED VIRGIN SOIL OR STRUCTURAL COMPACTED FILL PLACED OVER UNDISTURBED VIRGIN SOIL HAVING AN ALLOWABLE BEARING CAPACITY OF 2500 POUNDS PER SQUARE FOOT IN ACCORDANCE WITH "THE GEOTECNICAL ENGINEERING REPORT" DATED JULY11, 2023 BY TRIAD ENGINEERING, INC. CAPACITY OF FOOTING SUBGRADE SHALL BE INSPECTED AND VERIFIED BY A GEOTECHNICAL ENGINEER LICENSED IN THE STATE OF VIRGINIA PRIOR TO PLACING CONCRETE.
2. ELEVATIONS SHOWN ON THE DRAWINGS ARE TO THE BOTTOM OF FOUNDATIONS AND ARE MINIMUM DEPTHS. IF BEARING MATERIALS AT THE SPECIFIED ELEVATIONS ARE FOUND TO HAVE LOWER BEARING CAPACITIES THAN REQUIRED, MATERIALS SHALL BE REMOVED AND REPLACED WITH LEAN CONCRETE.
3. DESIGN, FURNISH, AND PLACE ALL TEMPORARY OR PERMANENT SUPPORTS, WHETHER SHORING, SHEETING, OR BRACING, SO THAT NO HORIZONTAL MOVEMENT OR VERTICAL SETTLEMENT OCCURS TO EXISTING STRUCTURES, STREETS, OR UTILITIES ADJACENT TO PROJECT SITE.
4. INITIAL SITE CLEARING AND GRUBBING SHOULD INCLUDE REMOVAL OF THE TOPSOIL AND ANY OTHER DELETERIOUS MATERIALS WITHIN THE NEW STRUCTURE FOOTPRINT AND PAVEMENT AREAS AND EXTENDING TEN (10) FEET BEYOND THEIR PERIMETERS.
5. THE EXISTING FILL MATERIALS BE REMOVED FROM THE NEW BUILDING FOOTPRINT TO A MINIMUM DEPTH OF 3 FEET BELOW THE DESIGN BEARING ELEVATION, EXTENDING AT LEAST 5 FEET BEYOND THE FOOTPRINT AND BE REPLACED WITH NEW CONTROLLED FILL. THE EXISTING FILL CAN BE RE-USED FOR NEW FILL IF ORGANICS ARE EXCLUDED, AND THE MATERIAL IS MOISTURE CONDITIONED TO ACHIEVE THE REQUIRED COMPACTION.
6. THE REMAINING MATERIAL SHOULD BE HEAVILY PROOF-ROLLED WITH APPROPRIATE EQUIPMENT TO RE-DENSIFY THE FILL AS WELL AS HELP IDENTIFY ANY UNSTABLE AREAS.
7. CONTROL SURFACE AND SUBSURFACE WATER DURING CONSTRUCTION SO THAT FOUNDATION WORK WILL BE PERFORMED IN DRY CONDITIONS AND ON UNDISTURBED SOIL.
8. THE CONTRACTOR SHOULD BE PREPARED TO IMPLEMENT, IF NECESSARY,TEMPORARY DE-WATERING MEASURES IN THESE AREAS DURING CONSTRUCTION. THESE MEASURES CAN INCLUDE SLOPING THE CUT AREAS TO APPROPRIATE SUMP PIT(S) AND PUMPING ACCUMULATED SURFACE RUNOFF FROM PRECIPITATION. ALL CUT AREAS SHOULD BE SEALED AT THE END OF EACH DAY, TO THE EXTENT WHICH CONSTRUCTION PRACTICALITY WILL PERMIT, TO HELP PREVENT INFILTRATION OF PRECIPITATION AND SUBSEQUENT UNSUITABLE SOIL CONDITIONS.
9. DUE TO THE FINE-GRAINED SOILS AT THE SITE, CONSTRUCTION DURING THE WINTER AND SPRING MONTHS IS OFTEN FUTILE. THIS WILL BE VERY DEPENDENT UPON SEASONAL CONDITIONS AT THE TIME OF EARTHWORK CONSTRUCTION. ALSO, THE FINE-GRAINED SOILS ARE RELATIVELY SENSITIVE TO MOISTURE FLUCTUATIONS AND TYPICALLY CAN BE EFFECTIVELY PLACED AND COMPACTED ONLY DURING DRIER SEASONS.
10. FILL MATERIALS SHOULD NOT CONTAIN ANY DEBRIS, WASTE, OR FROZEN MATERIALS AND THEY SHOULD CONTAIN LESS THAN TWO (2) PERCENT VEGETATION-ORGANIC MATERIALS BY WEIGHT. ALSO, MATERIALS CLASSIFIED AS OL, OH, OR PT ARE NOT SUITABLE FOR USE AS STRUCTURAL FILL. THE ONSITE CLAYEY SOILS ARE GENERALLY SUITABLE FOR RE-USE AS STRUCTURAL FILL PROVIDED THAT PROPER DRAINAGE, GRADING AND SLOPING AWAY FROM THE STRUCTURE IS MAINTAINED BOTH DURING AND AFTER CONSTRUCTION.
11. BEFORE COMPACTION, EACH LAYER SHOULD BE MOISTENED OR AERATED AS NECESSARY TO OBTAIN THE REQUIRED COMPACTION. EACH LAYER SHOULD BE COMPACTED TO THE REQUIRED PERCENTAGE OF MAXIMUM DRY DENSITY. FILL SHOULD NOT BE PLACED ON SURFACES THAT ARE MUDDY OR FROZEN OR HAVE NOT BEEN APPROVED BY TESTING AND/OR PROOF-ROLLING. FREE WATER SHOULD BE PREVENTED FROM APPEARING ON THE SURFACE DURING OR SUBSEQUENT TO COMPACTION OPERATIONS.
12. EXCAVATIONS FOR FOOTINGS SHALL BE FINISHED BY HAND.
13. FOUNDATION CONCRETE SHALL NOT BE PLACED IN WATER OR ON FROZEN GROUND.
14. ALL STRUCTURAL COMPACTED FILL SHALL CONSIST OF CLEAN, WELL- GRADED GRANULAR MATERIAL CONTAINING NO MORE THAN 12% NOR LESS THAN 5% BY WEIGHT OF MATERIAL PASSING THE #200 SIEVE. MATERIAL SHALL BE FREE FROM CLAY LUMPS, ORGANICS AND DELETERIOUS MATERIAL. EXISTING ON SITE FILL/EXCAVATED MATERIAL MAY BE USED FOR BACKFILLING PROVIDED IT IS INSPECTED BY THE SOILS ENGINEER AND MEETS THE CRITERIA ABOVE.
15. ALL STRUCTURAL COMPACTED FILL AND BACKFILL IN BUILDING AND WITHIN 5'-0" OF BUILDING SHALL BE PLACED IN 12" MAXIMUM LOOSE LIFTS AND COMPACTED WITH A HEAVY VIBRATORY COMPACTOR TO AT LEAST 95% OF THE STANDARD PROCTOR DENSITY AS PER ASTM D-698 UNDER THE SUPERVISION OF A LICENSED SOILS ENGINEER.
16. ALL FILL AND BACKFILL SHALL BE PLACED ON VIRGIN SOIL THAT DOES NOT CONTAIN ANY ORGANIC MATERIAL. STRIP ALL TOP SOIL. PRIOR TO PLACING FILL OR BACKFILL, PROOF-COMPACT SUBGRADE WITH A HEAVY VIBRATORY COMPACTOR TO AT LEAST 95% OF THE MAXIMUM STANDARD PROCTOR DENSITY AS PER ASTM D-698 UNDER THE SUPERVISION OF A LICENSED SOILS ENGINEER.
17. ANY HARD ROCK WHICH IS ENCOUNTERED ABOVE THE PLANNED BEARING ELEVATION SHOULD BE OVER EXCAVATED TO AT LEAST 12 INCHES BELOW BEARING LEVELS AND SHOULD BE REPLACED WITH CONTROLLED SOIL FILL.
18. CRUSHED STONE BASE FOR SLAB ON GRADE SHALL BE A CLEAN, FINE-GRADED MATERIAL WITH A MAXIMUM PARTICLE SIZE OF 3/4" AND AT LEAST 10%-30% PASSING A NO. 100 SIEVE. CRUSHED STONE SHALL CONTAIN NO CLAY, SILT, OR ORGANIC MATERIAL.
19. FOUNDATION ELEMENTS SHALL BE CONSIDERED CENTERED UNDER COLUMN CENTERLINES UNLESS OTHERWISE NOTED.
20. NO FOOTINGS SHALL BE PLACED ABOVE 1 VERTICAL ON 2 HORIZONTAL SLOPE EXTENDED FROM THE CLOSEST EDGE OF ANY UNDISTURBED SOIL OR OTHER FOUNDATION STRUCTURE. BOTTOM OF EXTERIOR FOOTINGS SHALL NOT BE LESS THAN 2'-6" BELOW FINISHED GRADE.
21. FOUNDATION CONCRETE SHOULD BE PLACED THE SAME DAY THAT EXCAVATIONS ARE COMPLETED TO REDUCE THE POTENTIAL FOR SOFTENING DUE TO PRECIPITATION AND/OR RUNOFF.
22.
23. WHERE SOFT AREAS ARE ENCOUNTERED, THE AREA SHALL BE UNDERCUT AS DIRECTED BY THE SOILS ENGINEER AND REPLACED WITH COMPACTED FILL OR LEAN CONCRETE.
24. WHERE SHALLOW ROCK IS ENCOUNTERED AT FOOTING BEARING ELEVATION, THE ROCK SHALL BE REMOVED A MINIMUM OF 12" BELOW THE BOTTOM OF FOOTING AND REPLACED WITH CONTROLLED COMPACTED FILL.

CONCRETE CONSTRUCTION NOTES

1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE A.C.I. "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318).
2. CONCRETE DESIGN MIXES SHALL CONFORM WITH ASTM C94, AND HAVE PROPERTIES AS INDICATED BELOW:

   FOUNDATIONS, WALLS, AND PIERS: f'c=4,000 psi AT 28 DAYS
   MAX. W/C RATIO: 0.50
   AIR CONTENT: 5% ± 1 1/2%

   SLABS-ON-GRADE: f'c=4,000 psi AT 28 DAYS
   MAX. W/C RATIO: 0.50
   AIR CONTENT: 3% MAX.

3. SLUMP SHALL BE LIMITED TO 4 INCHES. FOR CONCRETE WITH HRWR (SUPER-P), SLUMP SHALL BE LIMITED TO 2-4 INCHES PRIOR TO ADDITION OF HRWR, AND A MAXIMUM OF 8 INCHES AFTER ADDITION OF HRWR.
4. ADMIXTURES USED IN CONCRETE SHALL BE AS ALLOWED BY THE SPECIFICATIONS AND ONLY WITH LABORATORY DESIGN MIX APPROVAL. ALL ADMIXTURES SHALL CONTAIN NO MORE CHLORIDE IONS THAN ARE PRESENT IN MUNICIPAL DRINKING WATER.
5. CONCRETE MATERIALS SHALL BE AS INDICATED BELOW:
   A. PORTLAND CEMENT: ASTM C150, TYPE I/II
   B. FLY ASH: ASTM C618 - 15% - 25% OF CEMENTITOUS MATERIAL
   C. NORMAL-WEIGHT AGGREGATES: ASTM C33, 3/4" MAXIMUM
   D. WATER: ASTM C94 AND POTABLE
6. ADMIXTURES SHALL BE AS INDICATED BELOW:
   A. AIR-ENTRAINING ADMIXTURE: ASTM C260
   B. WATER REDUCING ADMIXTURE: ASTM C494, TYPE A
   C. WATER REDUCING AND RETARDING ADMIXTURE: ASTM C494 TYPE D
   D. WATER-REDUCING, ACCELERATING ADMIXTURE: ASTM C494 TYPE E
   E. HIGH RANGE WATER REDUCING ADMIXTURE (SUPER-PLASTICIZER): ASTM C494, TYPE F
   F. HIGH RANGE WATER REDUCING AND RETARDING ADMIXTURE: ASTM C494 TYPE G
7. EPOXY JOINT FILLER SHALL BE A TWO-COMPONENT SEMI RIGID RESIN, 100% SOLIDS, AND HAVE A MINIMUM SHORE A HARDNESS OF 80 WHEN MEASURED IN ACCORDANCE WITH ASTM D 2240.
8. ALL REINFORCING STEEL SHALL BE INTERMEDIATE GRADE, NEW BILLET STEEL, DEFORMED BARS, CONFORMING TO ASTM A-615, GRADE 60. ALL BARS SHALL BE SECURELY SUPPORTED AND WIRED IN PLACE PRIOR TO CONCRETE PLACEMENT.
9. ALL WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM A-185.
10. FIBER REINFORCING SHALL BE MONOFILAMENT POLYPROPYLENE FIBERS FOR SECONDARY REINFORCEMENT, ASTM C1116, TYPE III.
11. VAPOR RETARDER SHALL CONFORM TO ASTM E1745, CLASS C, WITH MINIMUM 10 MIL. THICKNESS.

CONCRETE CONSTRUCTION NOTES (CONT'D)

12. REINFORCING STEEL SHOWN IN SECTIONS ARE SCHEMATIC INDICATIONS THAT REINFORCING EXISTS. SEE SECTION NOTES, SCHEDULES, PLAN NOTES, ETC. FOR ACTUAL REINFORCING REQUIRED.
13. UNLESS OTHERWISE NOTED, ALL BARS MARKED CONT. SHALL BE SPLICED AT ALL LAP POINTS AND CORNERS AND DEVELOPED AT NON-CONTINUOUS ENDS AS TYPICAL DETAILS. SPLICE CONTINUOUS TOP BARS AT CENTER BETWEEN SUPPORTS AND SPLICE CONTINUOUS BOTTOM BARS AT SUPPORTS. WELDED WIRE FABRIC SHALL BE LAPPED 12 INCHES OR TWO SPACES, WHICHEVER IS LONGER. SHEETS SHALL BE WIRED TOGETHER.
14. CONCRETE COVER FOR REINFORCING BARS SHALL BE AS SHOWN IN DETAILS.
15. SLAB ON GRADE SHALL BE WET CURED WITH A MOISTURE RETAINING COVER CONFORMING TO ASTM C171 WITH SIDES AND ENDS LAPPED AT LEAST 12", SEALED WITH WATERPROOF TAPE. MOISTURE RETAINING COVER SHALL REMAIN IN PLACE FOR A MINIMUM OF 7 DAYS.
16. AT OPENINGS IN CONCRETE WALLS, PROVIDE ADDED REINFORCEMENT IN ACCORDANCE WITH THE TYPICAL DETAILS UNLESS OTHERWISE NOTED.
17. REINFORCEMENT SHALL NOT BE WELDED OR HEATED IN ANY WAY.
18. SLEEVES, MECHANICAL OPENINGS, CONDUITS, PIPES, RECESSES, DEPRESSIONS, CURBS, AND ALL EMBEDDED ITEMS SHALL BE PROVIDED FOR AS SHOWN ON THE ARCHITECTURAL AND MECHANICAL DRAWINGS AND AS REQUIRED BY EQUIPMENT MANUFACTURERS. MINIMUM CONCRETE BETWEEN SLEEVES SHALL BE 6". INSTALLATION OF THESE ITEMS SHALL BE COORDINATED WITH SHOP DRAWINGS OF TRADES REQUIRING THESE ITEMS.
19. SET FORMS TO FOLLOW SLOPES AND GRADES DEFINED ON PLAN, KEEPING MEMBER DEPTHS CONSTANT AS DETAILED OR SCHEDULED, UNLESS NOTED OTHERWISE. SLOPE UNIFORMLY BETWEEN ELEVATIONS GIVEN.
20. REINFORCING, INCLUDING WELDED WIRE FABRIC, FOR SLABS ON GRADE AND FOOTINGS SHALL BE SUPPORTED ON SOLID CONCRETE BLOCKS AT 5'-0" ON CENTER MAXIMUM EACH WAY. REINFORCING, INCLUDING WELDED WIRE FABRIC, FOR OTHER SLABS SHALL BE SUPPORTED ON CHAIRS AND BOLSTERS AT ALL SUPPORTS AND AT 5'-0" ON CENTER MAXIMUM BETWEEN SUPPORTS.
21. VERTICAL CONSTRUCTION JOINTS IN CONCRETE WALLS SHALL BE LOCATED AT MIDPOINT BETWEEN ANY SUPPORTING PIERS OR BUTTRESSES, AND AT LEAST 4'-0" FROM ANY WALL OPENING EXCEPT WHERE SPECIFICALLY SHOWN ON THE DRAWINGS. HORIZONTAL CONSTRUCTION JOINTS SHALL NOT BE PERMITTED, EXCEPT WHERE SHOWN ON DETAILS.
22. PROVIDE SHEAR KEY IN ALL CONSTRUCTION JOINTS IN WALLS.
23. ALL CONSTRUCTION JOINTS SHALL BE THOROUGHLY CLEANED AND TREATED WITH THE SPECIFIED BONDING COMPOUND JUST BEFORE PLACING NEW CONCRETE.
24. SEE ARCHITECTURAL DRAWINGS FOR DETAILS OF WEEPHOLES, FLASHING REGLETS, FASCIA DETAILS, ETC..
25. UNDER NO CIRCUMSTANCES SHALL CONCRETE BE PUMPED THROUGH ALUMINUM PIPES. CONCRETE SHALL NOT BE PLACED IN CONTACT WITH ALUMINUM, ALUMINUM MIXING DRUMS, TRUCK MIXERS, BUGGLES, CHUTES, CONVEYORS, TREMIE PIPES, AND OTHER EQUIPMENT MADE OF ALUMINUM SHALL NOT BE USED ON THIS PROJECT.
26. WHERE CONCRETE ABUTS MASONRY, PROVIDE VERTICAL METAL SLOTS TO RECEIVE GALVANIZED METAL DOVETAIL ANCHORS. SLOTS SHALL BE SPACED AT 24" ON CENTER.
27. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR FABRICATION, BENDING, AND PLACEMENT OF CONCRETE REINFORCEMENT. SHOP DRAWINGS SHALL COMPLY WITH ACI 315 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES".
28. ALL CONCRETE REINFORCING IS SUBJECT TO INSPECTION BY THE DESIGN ENGINEER PRIOR TO CONCRETE PLACEMENT. CONTRACTOR SHALL NOTIFY ENGINEER OF ANY DISCREPANCY FROM ACI 315 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES".
29. COLD OR HOT WEATHER CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH THE APPLICABLE CODE REQUIREMENTS.
30. INSTALLATION OF POST-INSTALLED ADHESIVE ANCHORS MUST BE INTO CONCRETE THAT HAS A MINIMUM AGE OF 21 DAYS AT THE TIME OF INSTALLATION.

STEEL CONSTRUCTION NOTES

1. ALL STRUCTURAL STEEL WORK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST AISC CODE OF STANDARD PRACTICE. STRUCTURAL STEEL SHALL BE NEW, CLEAN, AND STRAIGHT, AND SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
   A. WIDE FLANGE ROLLED SHAPES: ASTM A992, GRADE 50 (Fy = 50 KSI).
   B. PLATES, ANGLES, BARS, CHANNELS, AND S SHAPES: ASTM A36 (Fy = 36 KSI).
   C. RECTANGULAR HSS: ASTM A500, GRADE B (Fy = 46 KSI).
   D. ROUND HSS: ASTM A500, GRADE B (Fy = 42 KSI).
   E. PIPE: ASTM A53, TYPE E OF S, GRADE B (Fy = 35 KSI).
2. ALL ANCHOR RODS, UNLESS OTHERWISE NOTED, SHALL BE ASTM F1554, GRADE 36.
3. ALL BOLTED CONNECTIONS, UNLESS OTHERWISE NOTED, SHALL BE 3/4"Ø A325 HIGH STRENGTH BOLTS, IN BEARING TYPE CONNECTIONS AND SHALL BE PROVIDED WITH HARDENED WASHERS UNDER THE TURNED ELEMENT (NUT OR BOLT THREAD).
4. ALL STRUCTURAL STEEL SHALL BE PAINTED WITH ONE COAT OF SHOP PRIMER. THE EXCEPTIONS INCLUDE WHERE FIELD WELDING OR SLIP CRITICAL BOLTING IS TO BE DONE, WHERE STEEL IS TO RECEIVE SPRAY-ON FIREPROOFING, WHERE STEEL IS TO BE EMBEDDED IN CONCRETE, AND WHERE STEEL IS TO BE HOT-DIPPED GALVANIZED.
5. STRUCTURAL STEEL EXPOSED TO WEATHER, EXCESSIVE MOISTURE, OR CORROSIVE ENVIRONMENT AND AS INDICATED ON CONSTRUCTION DOCUMENTS, SHALL BE HOT-DIPPED GALVANIZED, MEETING REQUIREMENTS OF ASTM A123 AND A153 AS APPLICABLE.
6. INSTALLATION AND TIGHTENING OF ALL HIGH STRENGTH BOLTS SHALL CONFORM TO THE AISC "SPECIFICATION FOR THE STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."
7. CONNECTIONS MAY BE WELDED OR HIGH STRENGTH BOLTED. ALL CONNECTIONS SHALL CONFORM TO THE TYPICAL CONNECTION DETAILS SHOWN ON THE DRAWINGS.
8. ALL WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY STRUCTURAL WELDING CODE - STEEL (AWS D1.1) AND SHALL BE DONE BY A.W.S. QUALIFIED WELDERS USING E70XX ELECTRODES.
9. ALL CONTACT SURFACES WITHIN HIGH STRENGTH BOLTED CONNECTIONS AND WELDING AREAS SHALL BE FREE OF OIL, PAINT, AND LACQUER.
10. THE CONTRACTOR SHALL COORDINATE THE SIZE AND LOCATION OF ALL ROOF OPENINGS SHOWN ON THE STRUCTURAL, ARCHITECTURAL AND/OR MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. ANY STEEL WHICH IS NOT SHOWN ON THE CONTRACT DRAWINGS AS FURNISHED BY THE STRUCTURAL STEEL CONTRACTOR AND WHICH IS REQUIRED BY THE MECHANICAL, PLUMBING, AND ELECTRICAL TRADES FOR OPENINGS AND/OR TO SUPPORT THEIR WORK SHALL BE SUPPLIED AND INSTALLED BY THE CONTRACTOR REQUIRING SUCH STEEL, UNLESS OTHERWISE NOTED.
11. CUTS, HOLES, COPING, ETC. REQUIRED IN STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES SHALL BE SHOWN ON THE STRUCTURAL STEEL SHOP DRAWINGS AND BE MADE IN THE SHOP. HOLES SHALL BE REINFORCED AND APPROVED BY THE STRUCTURAL ENGINEER.
12. BURNING OF HOLES, CUTS, ETC. IN STRUCTURAL STEEL MEMBERS IN THE FIELD WILL NOT BE PERMITTED, EXCEPT WITH THE SPECIFIC WRITTEN PERMISSION OF THE ENGINEER.
13. FOR MISCELLANEOUS STEEL, SEE ARCHITECTURAL DRAWINGS.
14. SUBMIT ALL STRUCTURAL STEEL SHOP DRAWINGS FOR REVIEW PRIOR TO ANY FABRICATION A MINIMUM OF TWO (2) WEEKS PRIOR TO THE START OF FABRICATION.

WOOD TRUSS CONSTRUCTION NOTES

1. ROOF TRUSSES SHALL BE PRE-MANUFACTURED WOOD TRUSSES DESIGN BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF VIRGINIA. TRUSS DESIGN DRAWINGS SHALL BE SIGNED AND SEALED BY THE DESIGN PROFESSIONAL ENGINEER AND SHALL INCLUDE, AT A MINIMUM, THE FOLLOWING:
   a. SLOPE OR DEPTH, SPAN AND SPACING
   b. LOCATIONS OF JOINTS
   c. REQUIRED BEARING WIDTHS
   d. DESIGN LOADS AS APPLICABLE
   e. TOP CHORD LIVE LOAD (INCLUDING SNOW LOADS)
   f. TOP CHORD DEAD LOAD
   g. BOTTOM CHORD LIVE LOAD
   h. BOTTOM CHORD DEAD LOAD
   i. CONCENTRATED LOADS AND THEIR POINTS OF APPLICATION AS APPLICABLE
   j. CONTROLLING WIND AND EARTHQUAKE LOADS AS APPLICABLE
   k. ADJUSTMENTS TO LUMBER AND METAL CONNECTOR PLATE DESIGN VALUE FOR CONDITIONS OF USE
   l. EACH REACTION FORCE AND DIRECTION
   m. METAL CONNECTOR PLATE TYPE, SIZE, THICKNESS OR GAGE, AND THE DIMENSIONED LOCATION OF EACH METAL CONNECTOR PLATE EXCEPT WHERE SYMMETRICALLY LOCATED RELATIVE TO THE JOINT INTERFACE
   n. LUMBER SIZE, SPECIES AND GRADE FOR EACH MEMBER
   o. CONNECTION REQUIREMENTS FOR:
   o.a. TRUSS TO TRUSS
   o.b. TRUSS PLY TO PLY
   o.c. FIELD SPLICES
   p. CALCULATED DEFLECTION RATIO AND MAXIMUM VERTICAL AND HORIZONTAL DEFLECTION FOR LIVE AND TOTAL LOAD AS APPLICABLE
   q. MAXIMUM AXIAL TENSILE AND COMPRESSION FORCES IN THE TRUSS MEMBERS
   r. REQUIRE PERMANENT INDIVIDUAL TRUSS MEMBER BRACING
2. DESIGN, MANUFACTURE AND QUALITY ASSURANCE OF METAL-PLATE- CONNECTED WOOD TRUSSES SHALL BE IN ACCORDANCE WITH TPI 1.

WOOD CONSTRUCTION NOTES

1. STRUCTURAL SAWN LUMBER, STRUCTURAL GLUED LAMINATED TIMBER, STRUCTURAL COMPOSITE LUMBER, AND FASTENERS ARE TO CONFORM TO THE "NATIONAL DESIGNS SPECIFICATION (NDS) FOR WOOD CONSTRUCTION USING ALLOWABLE STRESS DESIGN (ASD). ALL LUMBER SHALL BEAR THE GRADE THE GRADE MARK OF A GRADING RULES AGENCY APPROVED BY THE AMERICAN LUMBER STANDARD COMMITTEE.
2. DIMENSIONAL LUMBER SHALL BE DOUGLAS FIR LARCH (NORTH), CONFORMING TO THE FOLLOWING MINIMUM STRESS REQUIREMENTS:

2"-4" FRAMING (NO. 1/NO. 2):	5"x5" AND LARGER (NO. 1):
Fb - 850 psi	Fb - 1,200 psi
Ft - 500 psi	Ft - 825 psi
Fv - 180 psi	Fv - 170 psi
Fc_{perp} - 625 psi	Fc_{perp} - 625 psi
Fc_{par} - 1,400 psi	Fc_{par} - 1,000 psi
E - 1,600,000 psi	E - 1,600,000 psi

3. STRUCTURAL COMPOSITE LUMBER INDICATED LVL AND PSL ARE LAMINATED VENEER LUMBER AND PARALLEL STRAND LUMBER, RESPECTIVELY, AS MANUFACTURED BY iLEVEL, OR EQUAL, WITH THE FOLLOWING MINIMUM PROPERTIES:

LVL :	PSL BEAM:
Fb - 2,600 psi	Fb - 2,900 psi
Ft - 1,555 psi	Ft - 2,025 psi
Fv - 285 psi	Fv - 290 psi
Fc_{perp} - 750 psi	Fc_{perp} - 750 psi
Fc_{par} - 2,510 psi	Fc_{par} - 2,900 psi
E - 1,900,000 psi	E - 2,000,000 psi

   PSL COLUMN:
   Fb - 2,400 psi
   Ft - 1,755 psi
   Fv - 190 psi
   Fc_{perp} - 425 psi
   Fc_{par} - 2,500 psi
   E - 1,800,000 psi

4. GRADE LOSS RESULTING FROM WEATHERING, HANDLING, STORAGE, RESAWING, OR DIVIDING LENGTHS WILL BE CAUSE FOR REJECTION.
5. DO NOT NOTCH OR DRILL JOISTS, BEAMS, OR LOAD BEARING STUDS WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
6. ALL SILL PLATES ON EXTERIOR FOUNDATION WALLS AND EXTERIOR WALL SHEATHING LOCATED WITHIN 8" FROM EXPOSED EARTH SHALL BE PRESERVATIVE TREATED USING WATER-BORNE PRESERVATIVES IN ACCORDANCE WITH AWPA U1.
7. ROOF SHEATHING SHALL BE 3/4" APA RATED 48/24 C-D EXPOSURE I, AS DESIGNATED ON DRAWINGS.
8. EXTERIOR WALL SHEATHING SHALL BE 5/8" APA RATED 16/0 C-D, EXPOSURE 1 UNLESS OTHERWISE NOTED ON SHEAR WALL ELEVATION SCHEDULE, EXTERIOR WALL SHEATHING SHALL BE FASTENED WITH 10d COMMON NAILS AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS, 1 1/2" EMBEDMENT MINIMUM INTO STUD OR BLOCKING..
9. SHEATHING SHALL BE CONTINUOUS OVER THREE OR MORE SUPPORTS AND SHALL BE INSTALLED WITH FACE GRAIN PERPENDICULAR TO SUPPORTS WITH JOINTS STAGGERED.
10. ROOF SHEATHING SHALL BE FASTENED WITH 10d COMMON NAILS AT 6" O.C AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS, 1 1/2" EMBEDMENT MINIMUM IN TRUSS CHORD OR BLOCKING. PROVIDE BLOCKING AT ALL EDGES OF SHEATHING.
11. SHEATHING SHALL NOT BE LESS THAN 4' X 8', EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING.
12. BUILT UP COLUMNS SHALL BE FASTENED TOGETHER AS FOLLOWS, UNLESS OTHERWISE NOTED ON THE DRAWINGS:
   (2) 2x4 - 1 ROW 10d NAILS @ 6" O.C., STAGGERED, 1" MIN. EDGE DISTANCE
   (3) 2X4 - 1 ROW 30d NAILS @ 8" O.C., STAGGERED, 1 1/2" MIN. EDGE DISTANCE
   (2) 2x6 - 2 ROWS 10d NAILS @ 6" O.C., 1" MIN. EDGE DISTANCE
   (3) 2x6 - 2 ROWS 30d NAILS @ 8" O.C., 1 1/2" MIN. EDGE DISTANCE
13. NAILS AND STAPLES SHALL CONFORM TO ASTM F 1667. NAILS INDICATED ON PLANS, SECTIONS, AND NOTES ARE COMMON NAILS UNLESS OTHERWISE NOTED.
   8d - 0.131" x 2 1/2"
   10d - 0.148" x 3"
   16d - 0.162" x 3 1/2"
14. FASTENERS IN CONTACT WITH PRESERVATIVE TREATED WOOD SHALL BE HOT-DIPPED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE, OR COPPER.
15. METAL CONNECTORS CALLED OUT ON THE DRAWINGS ARE AS MANUFACTURED BY "SIMPSON STRONG-TIE COMPANY, INC." USE MANUFACTURER'S RECOMMENDED NAILING AT ALL METAL CONNECTORS. SUBSTITUTIONS WILL NOT BE ALLOWED WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.
16. PROVIDE SOLID BLOCKING AND CONTINUOUS STUDS TO BEARING LEVEL BELOW ALL KING AND JACK STUDS IN BEARING WALLS.
17. ALL CONNECTORS TO STRUCTURAL COMPOSITE LUMBER SHALL BE FASTENED TO THE WIDE FACE OF THE MEMBER ONLY AND NOT TO THE EDGES OF THE LUMBER STRANDS/VENEERS.

SHOP DRAWING SUBMITTALS

1. GENERAL CONTRACTOR (GC) SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR REVIEW OF ALL REQUIRED INFORMATION AS INDICATED IN THE DRAWINGS AND SPECIFICATIONS, PRIOR TO ANY FABRICATION.
2. NO PORTION OF THE STRUCTURAL DRAWINGS SHALL BE REPRODUCED FOR USE AS SHOP DRAWINGS.
3. ALL DIMENSIONS SHALL BE COORDINATED BY THE GC AND/OR THE DETAILER.
4. DETAILER SHALL USE THE SAME GRID IDENTIFICATIONS AS THOSE SHOWN ON THE CONTRACT DRAWINGS.
5. ALL SHOP DRAWINGS SHALL BE SUBMITTED PRIOR TO PROCEEDING WITH ANY ASSOCIATED WORK AND SHALL ALLOW FOR SUFFICIENT REVIEW TIME. SUBMIT A MINIMUM OF TWO (2) WEEKS PRIOR TO THE START OF FABRICATION.
6. SHOP DRAWINGS SHALL BE SUBMITTED WITH GC'S STAMP OF APPROVAL, CERTIFYING THE GC HAS COORDINATED AND VERIFIED ALL DIMENSIONS, MATERIALS, AND ANY ADDITIONAL INFORMATION AFFECTING STRUCTURAL WORK. THE GC'S REVIEW INCLUDES BUT IS NOT LIMITED TO COORDINATION AND VERIFICATION OF ACTUAL FIELD CONDITIONS, DIMENSIONS, ELEVATIONS, AND SUPPORTS AND OPENINGS FOR ACTUAL EQUIPMENT PURCHASED.
7. SHOP DRAWINGS NOT COMPLYING WITH THE ABOVE SHALL BE RETURNED FOR CORRECTION WITHOUT REVIEWING.
8. RESUBMITTED SHOP DRAWINGS SHALL INCLUDE ALL CHANGES ON THE DRAWINGS CLOUDED AND MARKED WITH REVISION TAG NUMBER.
9. GC SHALL NOT PROCEED WITH ANY WORK OR FABRICATION UNTIL THE SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED BY THE ENGINEER.

1. CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK USING THE CONTRACTOR'S BEST SKILL AND ATTENTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE AND HAVE CONTROL OVER CONSTRUCTION MEANS, METHODS TECHNIQUES, SEQUENCE, AND JOB SITE SAFETY
2. GC MUST PROVIDE & INSTALL ALL PRODUCTS PER PLANS. ONLY SUBSTITUTED PRODUCTS NEED TO BE SUBMITTED TO THE ARCHITECT FOR APPROVAL. UNAPPROVED SUBSTITUTIONS WILL BE REPLACED AT THE EXPENSE OF THE GC.
3. VERBAL REPRESENTATION HAS NO VALUE AND ALL REQUESTS TO CHANGE ANY PRODUCTS OR SPECIFICATIONS PER PLANS, MUST BE SUBMITTED IN WRITING TO THE ARCHITECT & TLE FOR APPROVAL.

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PROFESSIONAL CERTIFICATION
NAME OF LICENSEE: SHANNON CROWNOVER
LICENSE NUMBER: 0402 06 6228

Project Number: TLEVA23-034	Scale: AS NOTED
Drawn By: R.E.	Approved By: SC

Drawing Name:
GENERAL NOTES

Drawing Number:
S-100

COMMONWEALTH OF VIRGINIA
SHANNON NICOLE CROWNOVER
Lic. No. 0402066228
PROFESSIONAL ENGINEER

2021.Q4.01 RETAIL PROTOTYPE

H:\DRAWINGS\TLE- THE LEARNING EXPERIENCE\TLE- VIRGINIA\TLEVA23-034 - TLE WINCHESTERVA 2600 PLEASANT VALLEY WAY\CDS\TLEVA23-034-S-100-GENERAL NOTES.DWG RELNAJJAR PLOTTED: 12/19/2023

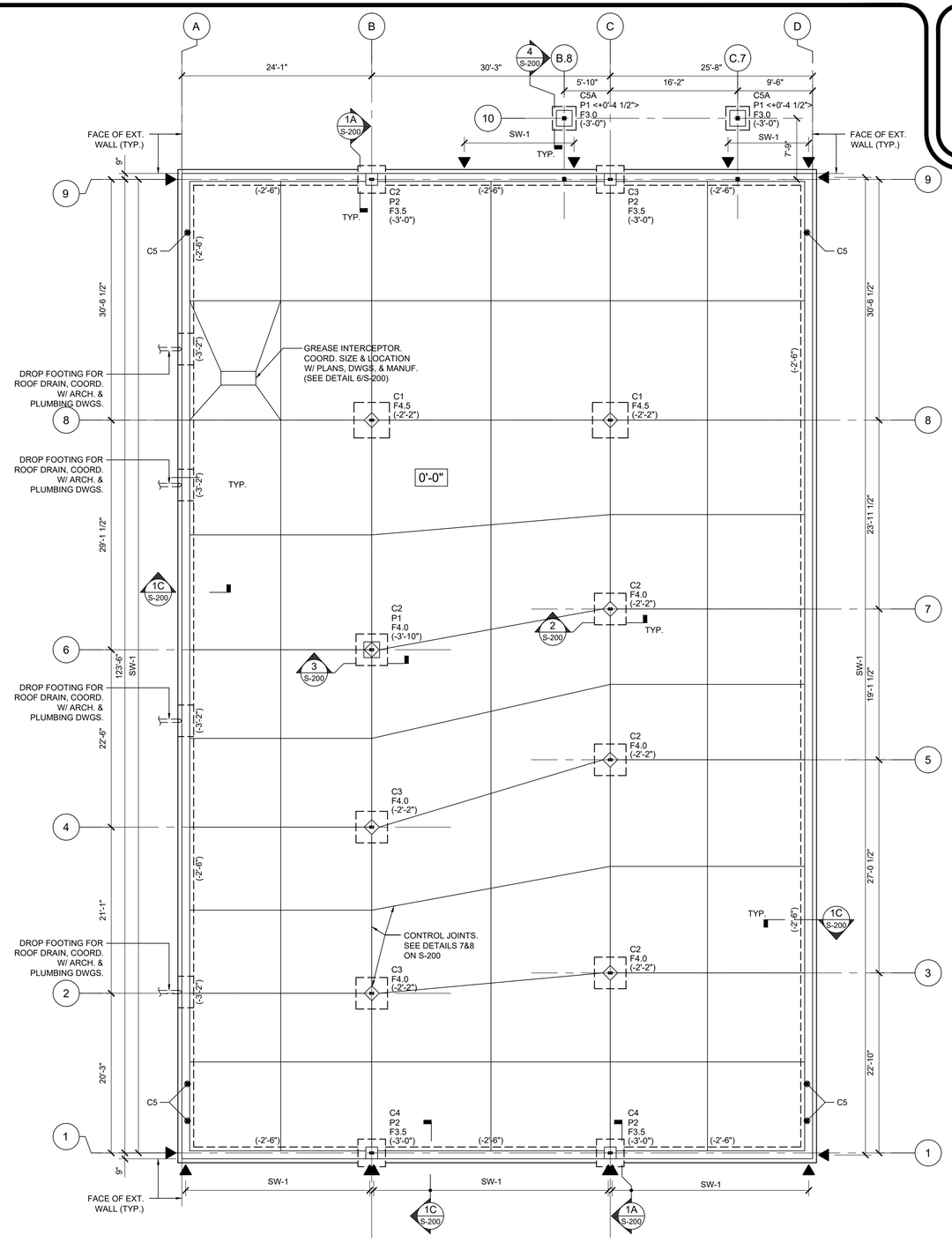
NOTES:

1. TOP OF SLAB IS SET AS DATUM 0'-0" AND IS ACTUAL ELEVATION 727.30'.
2. SLAB ON GRADE SHALL BE 4" NORMAL WEIGHT CONCRETE PLACED OVER A 10 MIL. VAPOR BARRIER ON 4" CRUSHED STONE. REINFORCE WITH 6x6-W1.4xW1.4 WELDED WIRE FABRIC OR 1.5 POUNDS FIBER REINFORCEMENT PER CUBIC YARD OF CONCRETE AT CONTRACTOR'S DISCRETION. PROVIDE SUBMITTAL FOR W.W.F. OR CONCRETE MIX DESIGN WITH FIBER REINFORCEMENT INCLUDED. SEE ARCHITECTURAL DRAWINGS FOR SCHEDULE OF FINISHES ON ALL EXPOSED CONCRETE SURFACES.
3. FOOTINGS ARE DESIGNED TO BEAR ON UNDISTURBED VIRGIN SOIL OR STRUCTURAL COMPACTED FILL PLACED OVER UNDISTURBED VIRGIN SOIL HAVING A MINIMUM ALLOWABLE BEARING CAPACITY OF 2500 PSF.
4. BOTTOM OF FOOTING ELEVATIONS ARE NOTED THUS (-X'-X") ON PLAN, REFERENCED FROM DATUM. ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM 2'-6" BELOW FINISHED GRADE. TOP OF COLUMN FOOTINGS TO ALIGN WITH TOP OF WALL FOOTINGS.
5. PX - INDICATES PIER TYPE, SEE PIER SCHEDULE FOR SIZE AND REINFORCING. TOP OF PIER ELEVATIONS ARE -0'-8" UNLESS NOTED THUS <-X'-X"> ON PLAN, REFERENCED FROM DATUM.
6. FX.X - INDICATES COLUMN FOOTING TYPE, SEE FOOTING SCHEDULE FOR SIZE AND REINFORCING.
7. SW-X INDICATES SHEAR WALL LOCATION, SEE SHEAR WALL SCHEDULE ON S-103.
8. ▶ INDICATES SHEAR WALL HOLDDOWN LOCATION, SEE SHEAR WALL SCHEDULE ON S-103 FOR SIZE AND ANCHORAGE.
9. FOR GENERAL NOTES, SEE DRAWING S-100.
10. FOR TYPICAL DETAILS, SEE DRAWING S-200.
11. * COORDINATE LOCATION OF POSTS WITH APPROVED ROOF TRUSS SHOP DRAWINGS.

PIER SCHEDULE					
MARK	SIZE		REINFORCING		REMARKS
	LENGTH	WIDTH	VERT.	TIES	
P1	20"	20"	4 - #7	#4 @ 12"	
P2	18"	18"	4 - #6	#4 @ 12"	

COLUMN SCHEDULE		
MARK	SIZE AND MATERIAL	BASE CONNECTION REQUIREMENTS
C1	HSS 8x3x3/8	BASE PL - 12"x9"x3/4" W/ (4) 3/4"Ø F1554 ANCHOR RODS, EMBED. 1'-0"
C2	HSS 6x3x1/2	BASE PL - 12"x9"x3/4" W/ (4) 3/4"Ø F1554 ANCHOR RODS, EMBED. 1'-0"
C3	HSS 6x3x1/2	BASE PL - 12"x9"x3/4" W/ (4) 3/4"Ø F1554 ANCHOR RODS, EMBED. 1'-0"
C4	HSS 6x3x1/4	BASE PL - 12"x9"x3/4" W/ (4) 3/4"Ø F1554 ANCHOR RODS, EMBED. 1'-0"
C5	5-1/4 x 5-1/4 PSL	BEAR ON WALL BOTTOM PLATE. ATTACH W/ DTT1Z W/ 3/8"Ø THREADED ROD W/ HILTI HIT HY200 ADHESIVE EMBED 3 3/8" MIN. INTO CONCRETE
C5A	5-1/4 x 5-1/4 PSL	SIMPSON ABU66Z W/ (12) 16d NAILS. ANCHOR TO CONC. W/ 5/8"Ø THREADED ROD IN HILTI HIT-HY200 ADHESIVE EMBED 5 5/8" AT CANOPY POSTS

FOOTING SCHEDULE 2500 PSF BEARING CAPACITY					
MARK	SIZE			BOTT. REINF. E.W. - EACH WAY L-LONG S-SHORT	TOP REINF. E.W. - EACH WAY L-LONG S-SHORT
	LENGTH	WIDTH	DEPTH		
F3.0	3'-0"	3'-0"	18"	7 - #4 E.W.	–
F3.5	3'-6"	3'-6"	18"	10 - #5 E.W.	–
F4.0	4'-0"	4'-0"	18"	11 - #5 E.W.	–
F4.5	4'-6"	4'-6"	18"	12 - #5 E.W.	–



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LICENSE NUMBER: 0402 06 6228

Project Number: TLEVA23-034	Scale: AS NOTED
Drawn By: R.E.	Approved By: SC

Drawing Name:

FOUNDATION PLAN

Drawing Number:

S-101

COMMONWEALTH OF VIRGINIA
SHANNON NICOLE CROWNOVER
Lic. No. 0402066228
PROFESSIONAL ENGINEER

2021.Q4.01 RETAIL PROTOTYPE

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NOTES:
1. TYPICAL ROOF CONSTRUCTION SHALL BE 3/4" SHEATHING OVER PRE-FABRICATED WOOD TRUSSES.
2. WOOD TRUSSES ARE SUPPORTED BY EXTERIOR BEARING WALLS AND INTERIOR STEEL BEAMS AND SHALL BE BOTTOM CHORD BEARING.
3. BOTTOM OF ROOF TRUSS BOTTOM CHORD (TOP OF BEARING PLATES) IS SET AT ELEVATION +11'-1 1/8". TOP OF ROOF TRUSS TOP CHORD SLOPES. WITH H.P. ELEVATION +14'-8 5/8".
4. ALL STUD WALLS SHOWN ON FRAMING PLAN ARE 2x6 AT 16" O.C. LOAD BEARING WALLS. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND DETAILS ON INTERIOR NON-LOAD BEARING PARTITION WALLS.
5. EXTERIOR WALLS ARE TO BE SHEATHED WITH 5/8" SHEATHING ANCHORED WITH 10d NAILS AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS, UNLESS OTHERWISE DESIGNATED ON THE SHEAR WALL SCHEDULE ON S-103.
6. ALL TRUSSES SHALL BE DESIGNED AND FABRICATED PER APPLICABLE LOADS BY THE TRUSS FABRICATOR. SUBMIT SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF VIRGINIA.
7. ROOF TRUSSES ARE TO BE A DEFERRED SUBMITTAL. SIGNED AND SEALED COPIES OF THE TRUSS CALCULATIONS AND THE PLACEMENT PLANS, WITH EVIDENCE THAT THE ENGINEER OF RECORD HAS REVIEWED AND APPROVED, SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT.
8. TRUSSES ARE TO BE DESIGNED FOR ALL LOADING AS DESIGNATED IN THE LOAD SCHEDULE AND ALL MECHANICAL UNITS SHOWN. COORDINATED LOCATION AND WEIGHTS OF UNITS WITH MECHANICAL DRAWINGS.
9. MECHANICAL EQUIPMENT DIMENSIONS SHOWN ON PLAN ARE TO CENTERLINE OF UNIT. DIMENSIONS TO BE VERIFIED WITH MECHANICAL DRAWINGS AND APPROVED HVAC SHOP DRAWINGS.
10. ALL TRUSSES/ROOF FRAMING ARE TO BE COORDINATED WITH MECHANICAL DRAWINGS FOR ROOF PENETRATIONS AND DUCTWORK LOCATIONS.
11. BM-X - INDICATES BEAM TYPE, SEE BEAM SCHEDULE FOR SIZE AND CONNECTION REQUIREMENTS. TOP OF BEAM ELEVATION NOTED THUS (+X'-X") ON PLAN, REFERENCED FROM BUILDINGS DATUM.
12. H-X - INDICATES HEADER TYPE, SEE HEADER SCHEDULE FOR SIZE AND CONNECTION REQUIREMENTS.
13. SW-X - INDICATES SHEAR WALL SEE SHEAR WALL SCHEDULE AND SHEAR WALL ELEVATION ON S-103 FOR MORE INFORMATION.
14. ALL BEAMS, HEADERS, AND TRUSSES ARE TO BEAR ON POSTS SHOWN ON PLAN WITH CONNECTIONS AS SPECIFIED IN THE COLUMN SCHEDULE.
15. SEE DRAWING S-100 FOR GENERAL NOTES.
16. UNLESS OTHERWISE NOTED ON PLAN, PROVIDE A (2) 2x6 POST AT ALL 2-PLY GIRDER TRUSS AND A (3) 2x6 POST AT ALL 3-PLY GIRDER TRUSS. GC SHALL COORDINATE LOCATION AND SIZE OF POST WITH APPROVED ROOF TRUSS SHOP DRAWINGS.

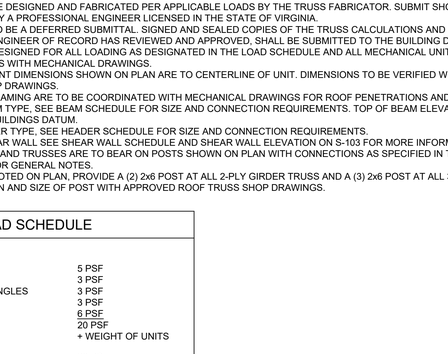
LOAD SCHEDULE

ROOF DEAD LOAD:	
WOOD TRUSSES	5 PSF
3/4" PLYWOOD	3 PSF
ROOFING/ASPHALT SHINGLES	3 PSF
CEILING/INSULATION	3 PSF
MEP/MISC.	6 PSF
TOTAL DEAD LOAD	20 PSF + WEIGHT OF UNITS
LIVE LOAD	20 PSF
APPLY 10 PSF OF DL TO TRUSS BOTTOM CHORD	
DESIGN FLAT ROOF SNOW LOAD	27 PSF*

* GROUND SNOW LOAD TO BE ADJUSTED FOR DRIFT, SLIDING, UNBALANCED LOADING, ETC. PER CODE.

HEADER SCHEDULE

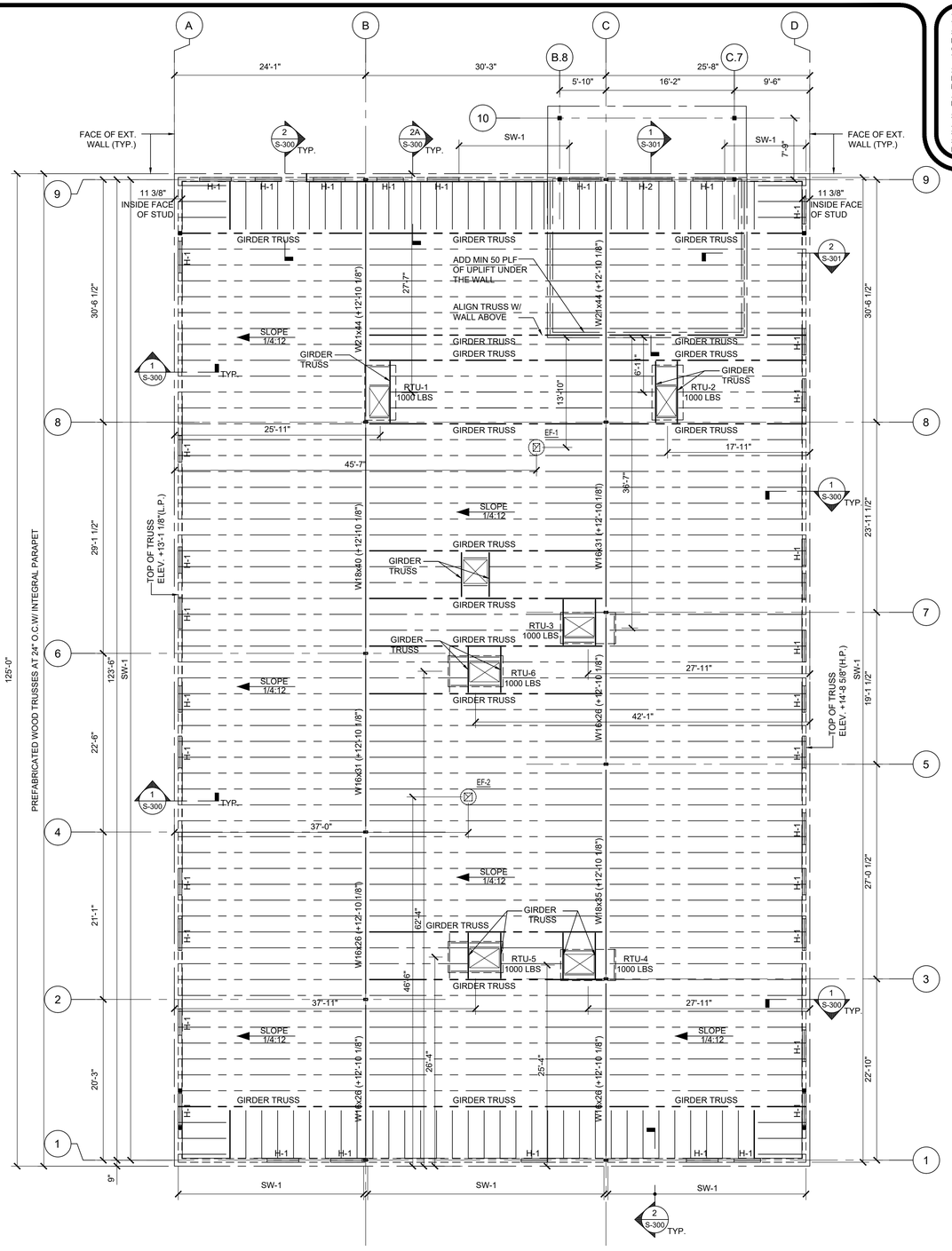
MARK	LOCATION	HEADER SIZE	JAMB MEMBER	HEADER CONNECTION
H-1	SINGLE DOOR / WINDOW 4'-0" MAXIMUM OPNG.	3-2x10 W/ 2-1/2" PLYWOOD SPACERS	2-2x6 EACH SIDE	SIMPSON A21 & LSTA9
H-2	DOUBLE DOOR / WINDOW 8'-0" MAXIMUM OPNG.	3-2x12 W/ 2-1/2" PLYWOOD SPACERS	2-2x6 EACH SIDE	SIMPSON A21 & LSTA9



CANOPY ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES:
1. TYPICAL ROOF CONSTRUCTION SHALL BE 3/4" SHEATHING OVER PRE-FABRICATED WOOD TRUSSES.
2. WOOD TRUSSES ARE SUPPORTED BY ROOF TRUSSES BELOW AND SHALL BE BOTTOM CHORD BEARING.
3. TOP OF ROOF TRUSS HIGH POINT TO BE ELEVATION +23-3 5/8". SLOPE TOP CHORD OF TRUSS MINIMUM 1/4" PER FOOT. TOP OF TRUSS LOW POINT TO BE ELEVATION +22-10 5/8".



ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

1. CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK USING THE CONTRACTOR'S BEST SKILL AND ATTENTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE AND HAVE CONTROL OVER CONSTRUCTION MEANS, METHODS TECHNIQUES, SEQUENCE, AND JOB SITE SAFETY
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LICENSE NUMBER: 0402 06 6228

Project Number: TLEVA23-034	Scale: AS NOTED
Drawn By: R.E.	Approved By: SC

Drawing Name:

ROOF FRAMING PLAN

Drawing Number:

S-102

COMMONWEALTH OF VIRGINIA
SHANNON NICOLE CROWNOVER
Lic. No. 0402066228
PROFESSIONAL ENGINEER

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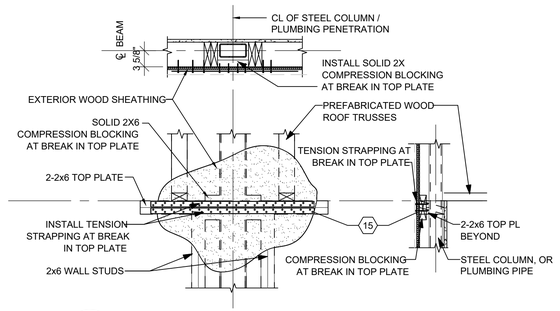
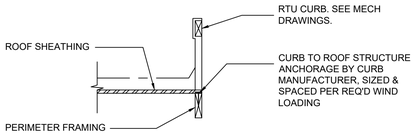
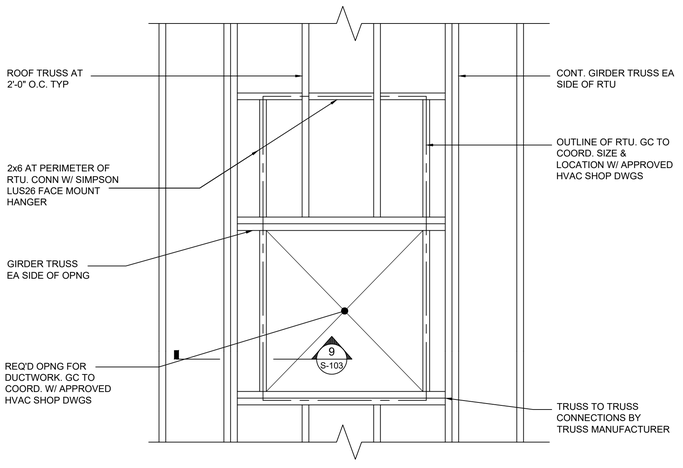


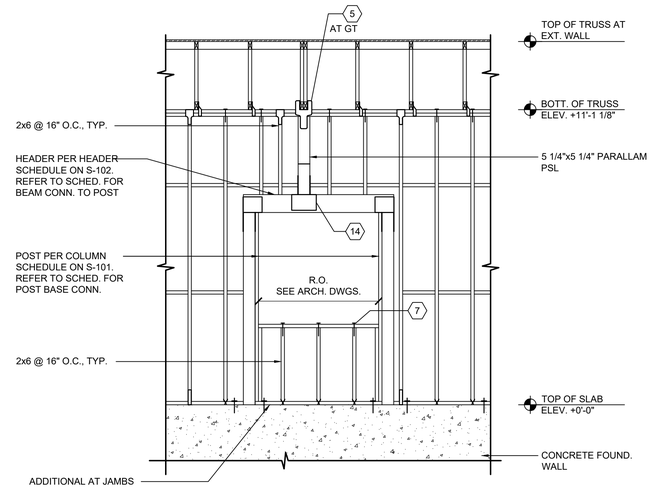
10 TYPICAL TOP PLATE TENSION TIE
SCALE : 3/4" = 1'-0"



9 TYPICAL RTU CURB SECTION
SCALE : 3/4" = 1'-0"



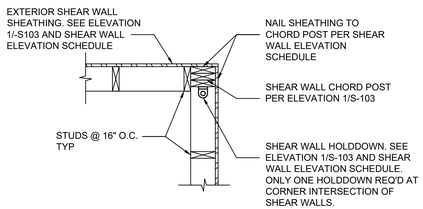
8 TYPICAL RTU CURB DETAIL
SCALE : 3/4" = 1'-0"



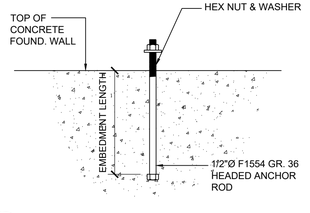
4 EXT. WALL OPNG BELOW GIRDER TRUSS
SCALE : 3/8" = 1'-0"

SHEAR WALL ELEVATION SCHEDULE		
#	ITEM DESCRIPTION	REMARKS
1	SIMPSON HDU2-SDS2.5 IN MINIMUM 3 INCH MEMBER W/ SIMPSON SB5/8X24 ANCHOR BOLT, EMBED 18 INCHES	AT LOCATION MARKED ON PLAN
2	1/2"Ø F1554 GR. 36 HEADED ANCHOR RODS @ 4 FT O.C., & WITHIN 12" OF SILL PL ENDS OR 1/2"Ø SIMPSON TITEN HD SCREW ANCHOR @ 4'-0" O.C. & WITHIN 12" OF SILL PL ENDS	9" EMBEDMENT FOR HEADED ANCHOR 3 5/8" EMBEDMENT FOR SCREW ANCHOR PROVIDE W/ 3" x 3" x 1/4" PL ON SILL FOR BOTH CAST-IN & POST-INSTALLED ANCHORS
3	SIMPSON H2.5T	(9) 10d X 1 1/2 TO TRUSS (6) 10d X 1 1/2 TO PLATES AT EACH TRUSS
4	NOT USED	
5	SIMPSON LGT2 (FOR 2-PLY GIRDER TRUSS) SIMPSON LGT3 (FOR 3-PLY GIRDER TRUSS)	LGT2- 14-16d SINKERS TO POST 16-16d SINKERS TO GIRDER TRUSS LGT3- 26-16d SINKERS TO POST 12-SDS 1/4"x2 1/2" TO GIRDER TRUSS
6	SIMPSON SP2	6-10d X 1 1/2" TO STUD, 6-10d PLATES EVERY OTHER STUD
7	2-16d COMMON NAILS	END NAIL TOP PLATE TO STUD, TYP.
8	4-8d COMMON NAILS	TOE NAIL STUD TO SOLE PLATE, TYP
9	SIMPSON SP1	6-10d X 1 1/2" TO STUD, 6-10d PLATES EVERY OTHER STUD. USE HDG NAILS MEETING ASTM A153
10	5/8" SHEATHING	EXTERIOR FACE ONLY
11	10d COMMON NAILS @ 6" O.C. AT LOCATIONS MARKED SW-1	AT PANEL EDGES. SPACING TO BE 12" O.C. AT INTERMEDIATE SUPPORTS, 1 1/2" EMBED. MIN. IN STUD/BLOCKING, TYP.
12	SIMPSON LSTA9	4-10d EACH TO STUD AND HEADER, 8-10d TOTAL
13	SIMPSON A21	2-10dx1 1/2 TO HEADER/ SILL 2-10dx1 1/2 TO JAMB
14	SIMPSON CCQ66SDS2.5	16 SDS 1/4"x2 1/2" SCREWS TO BEAM 14 SDS 1/4"x2 1/2" SCREWS TO POST
15	(2)CS20 W/ 6" END LENGTHS	(12) 0.148 x 2 1/2 FASTENERS AT EA. STRAP

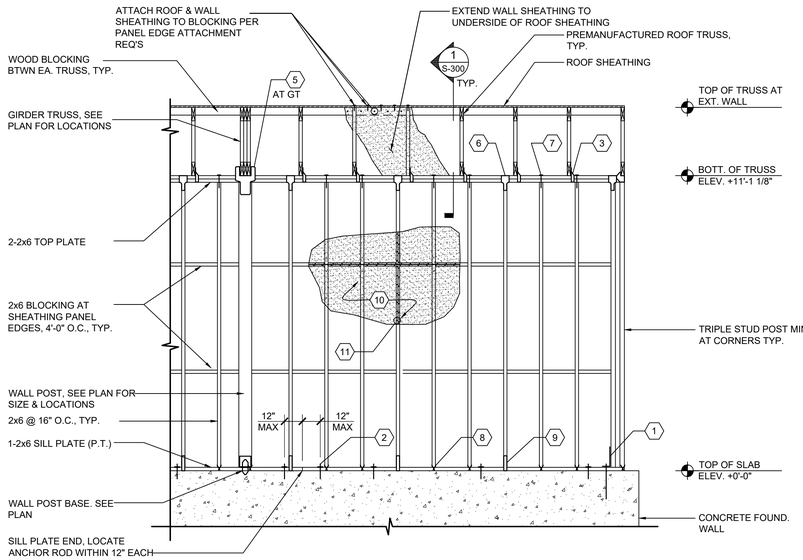
7 TOP PLATE SPLICE DETAIL
SCALE : 3/4" = 1'-0"



6 SHEAR WALL CORNER HOLDDOWN PLAN DETAIL
SCALE : 3/4" = 1'-0"

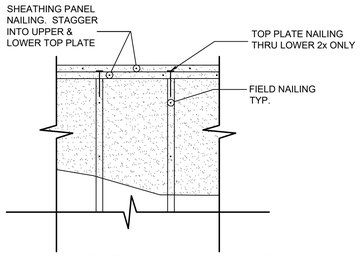


5 TYPICAL SILL ANCHOR DETAIL
SCALE : N.T.S.

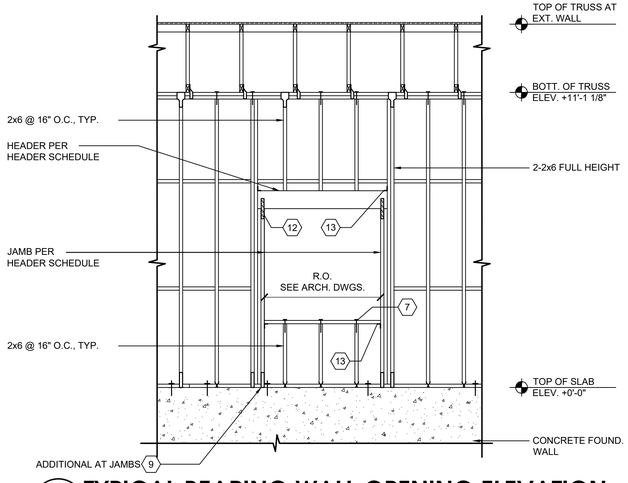


1 TYPICAL EXTERIOR SHEAR BEARING WALL ELEVATION
SCALE : 3/8" = 1'-0"

NOTE: HARDWARE LOCATIONS SHOWN ARE DIAGRAMATIC ONLY. SEE SHEAR WALL ELEVATION SCHEDULE FOR LOCATIONS AND SPACING



3 NAILING DETAIL AT TOP PLATE
SCALE : 3/4" = 1'-0"



2 TYPICAL BEARING WALL OPENING ELEVATION
SCALE : 3/8" = 1'-0"

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PROFESSIONAL CERTIFICATION
NAME OF LICENSEE: SHANNON CROWNOVER
LICENSE NUMBER: 0402 06 6228

Project Number: TLEVA23-034	Scale: AS NOTED
Drawn By: R.E.	Approved By: SC

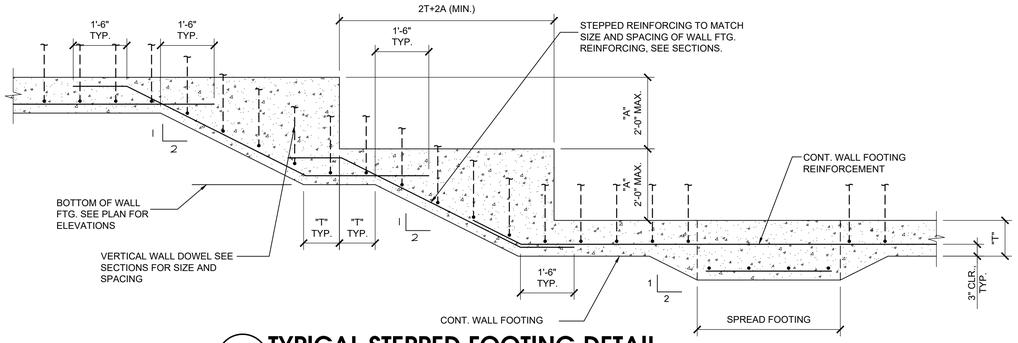
Drawing Name:
WALL ELEVATIONS

Drawing Number:
S-103

COMMONWEALTH OF VIRGINIA
SHANNON NICOLE CROWNOVER
Lic. No. 0402066228
PROFESSIONAL ENGINEER

2021.Q4.01 RETAIL PROTOTYPE

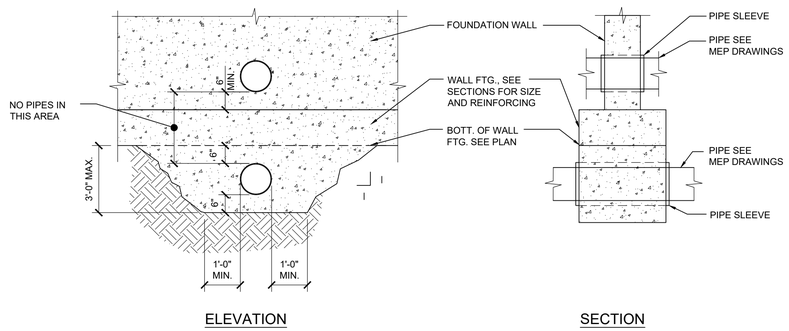
H:\DRAWINGS\TLE- THE LEARNING EXPERIENCE\TLE- VIRGINIA\TLEVA23-034 - TLE WINCHESTER\VA 2600 PLEASANT VALLEY WAY\CD\STLEVA23-034-S-103-WALL ELEVATIONS.DWG RELNAJJAR PLOTTED: 12/19/2023



11 TYPICAL STEPPED FOOTING DETAIL

SCALE : 1/2" = 1'-0"

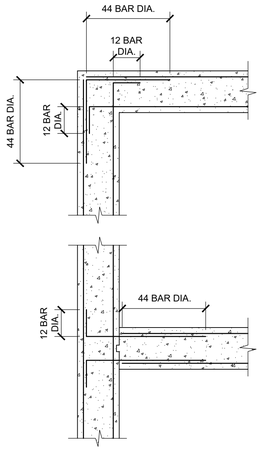
FOOTINGS FOR THIS PROJECT ARE NOT SHOWN ON PLAN TO BE STEPPED. THIS DETAIL IS ONLY TO BE USED SHOULD SITE CONDITIONS REQUIRE LOWERING/RAISING OF A PORTION OF THE FOOTING CREATING THE STEPPED FOOTING CONDITION.



ELEVATION SECTION

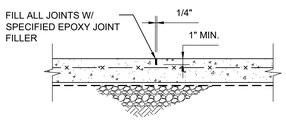
10 TYPICAL PIPE AT FOUNDATION WALL

SCALE : 1/2" = 1'-0"

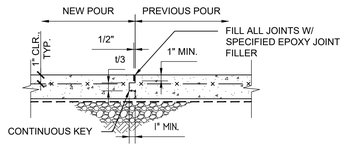


9 TYPICAL WALL INTERSECTION/ CORNER REINFORCING DETAIL

SCALE : 1/2" = 1'-0"



CONTROL JOINT DETAIL

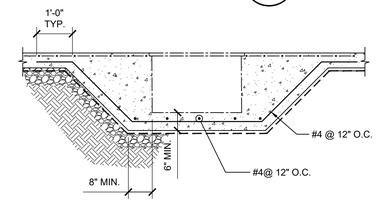


CONSTRUCTION JOINT DETAIL

NOTES:
1. CONTROL JOINTS SHALL BE LOCATED BY THE CONTRACTOR AT THE SPACING REQUIRED TO MEET SPECIFIED CRITERIA FOR MAXIMUM SLAB ON GRADE AREA BETWEEN CONTROL JOINTS.
2. LOCATE CONSTRUCTION JOINTS AT CONTROL JOINT LOCATIONS.
3. LOCATE JOINTS AS SHOWN ON THE DRAWINGS. MAXIMUM SPACING OF CONTROL JOINTS SHALL NOT EXCEED 20'-0" IN ANY DIRECTION.
4. SAW CUT JOINTS IMMEDIATELY AFTER SLAB RECEIVES FINAL STEEL TROWEL. ALL CUTTING RESIDUE SHALL BE REMOVED IMMEDIATELY FROM TOP OF CONCRETE SLAB.

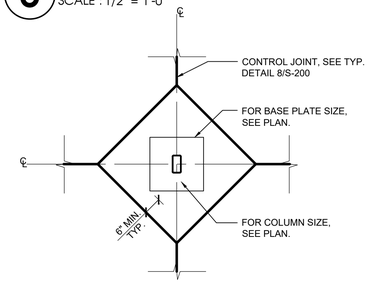
8 TYPICAL CONTROL JOINT DETAIL

SCALE : N.T.S.



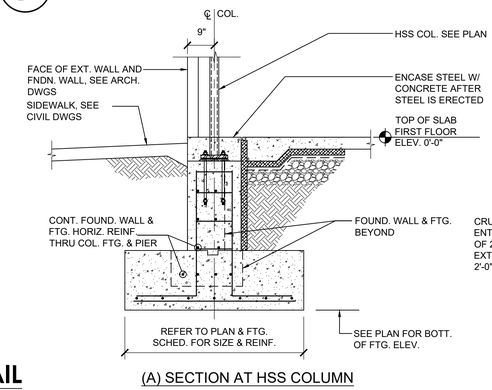
6 TYPICAL GREASE TRAP DETAIL

SCALE : 1/2" = 1'-0"

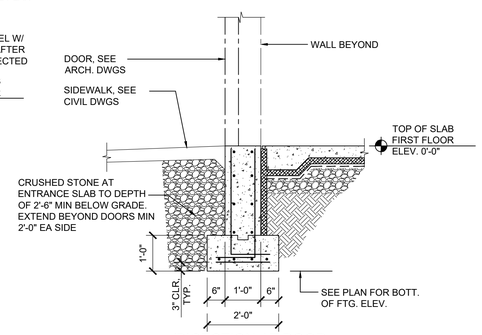


7 TYPICAL CONTROL JOINT DETAIL AT COLUMN

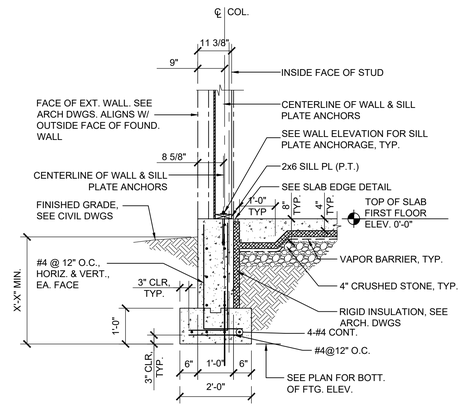
SCALE : 1/2" = 1'-0"



(A) SECTION AT HSS COLUMN



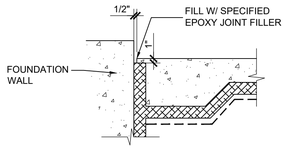
(B)SECTION AT DOOR



(C)TYPICAL SECTION

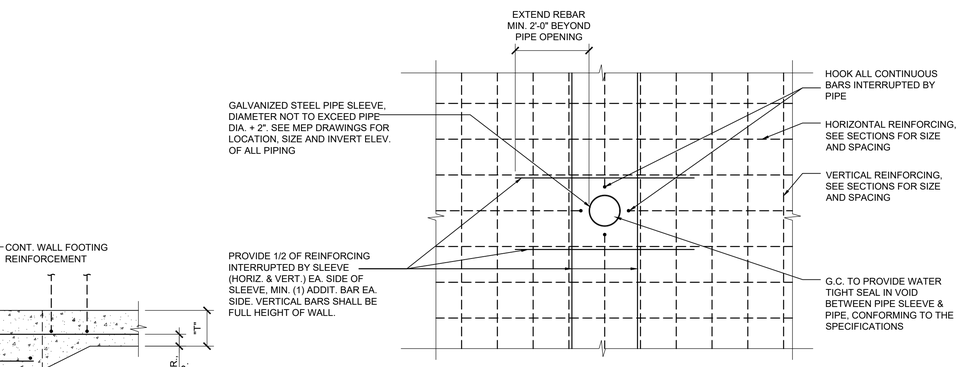
1 EXTERIOR WALL DETAIL

SCALE : 1/2" = 1'-0"



SLAB EDGE DETAIL

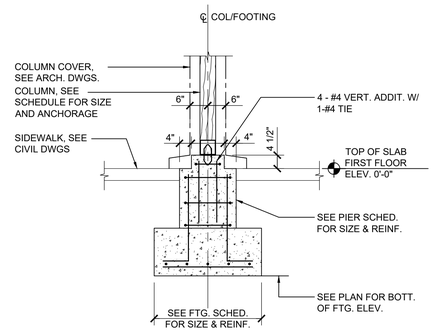
N.T.S.



NOTES:
1. HOOK ALL INTERRUPTED REINFORCING BARS
2. MINIMUM CLEARANCE BETWEEN SLEEVES SHALL BE 2'-0".

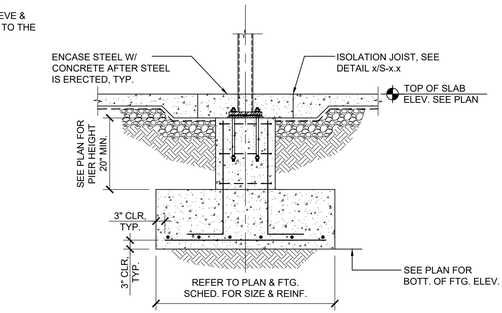
5 TYPICAL PIPE PENETRATION THRU CONC. FNDN WALL

SCALE : 1/2" = 1'-0"



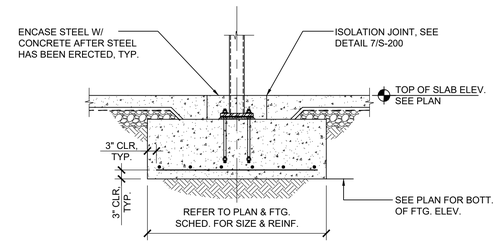
4 CANOPY COLUMN FOOTING DETAIL

SCALE : 1/2" = 1'-0"



3 TYPICAL INTERIOR COLUMN FOOTING WITH PIER DETAIL

SCALE : 1/2" = 1'-0"



2 TYPICAL INTERIOR COLUMN FOOTING DETAIL

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REVISION

NO.	DATE	DESCRIPTION	INT.

PROFESSIONAL CERTIFICATION
NAME OF LICENSEE: SHANNON CROWNOVER
LICENSE NUMBER: 0402 06 6228

Project Number: TLEVA23-034	Scale: AS NOTED
Drawn By: R.E.	Approved By: SC

Drawing Name:
FOUNDATION SECTIONS

Drawing Number:
S-200

COMMONWEALTH OF VIRGINIA
SHANNON NICOLE CROWNOVER
Lic. No. 0402066228
PROFESSIONAL ENGINEER

2021.Q4.01 RETAIL PROTOTYPE

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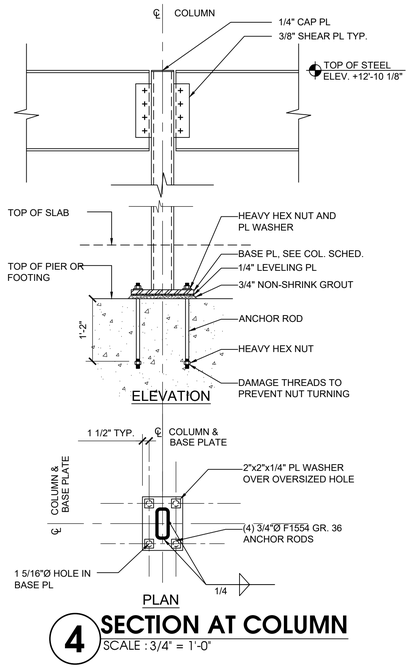
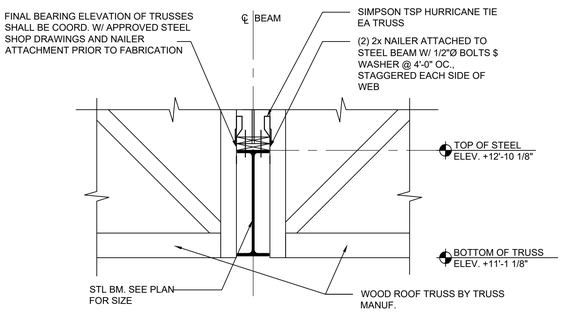
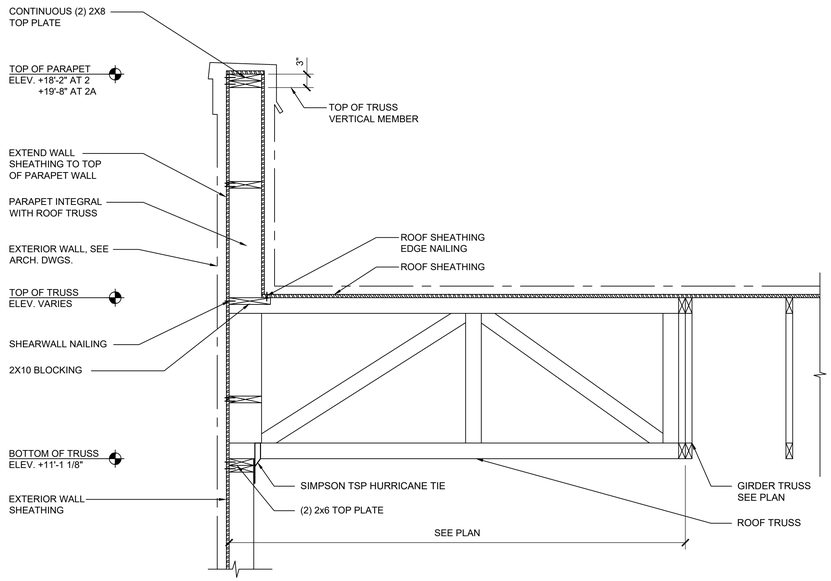


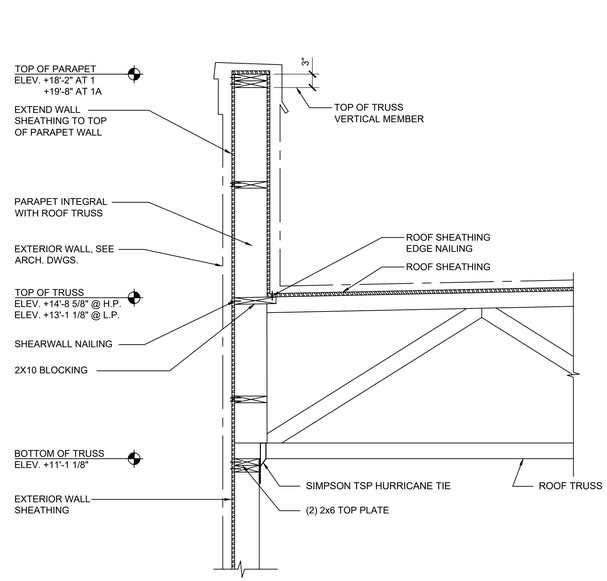
4 SECTION AT COLUMN
SCALE : 3/4" = 1'-0"



3 SECTION THRU STEEL BEAM
SCALE : 3/4" = 1'-0"



2 2A SECTION AT EXT. WALL BRG
SCALE : 3/4" = 1'-0"



1 SECTION AT EXT. WALL BRG
SCALE : 3/4" = 1'-0"

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LICENSE NUMBER: 0402 06 6228

Project Number: TLEVA23-034	Scale: AS NOTED
Drawn By: R.E.	Approved By: SC

Drawing Name:

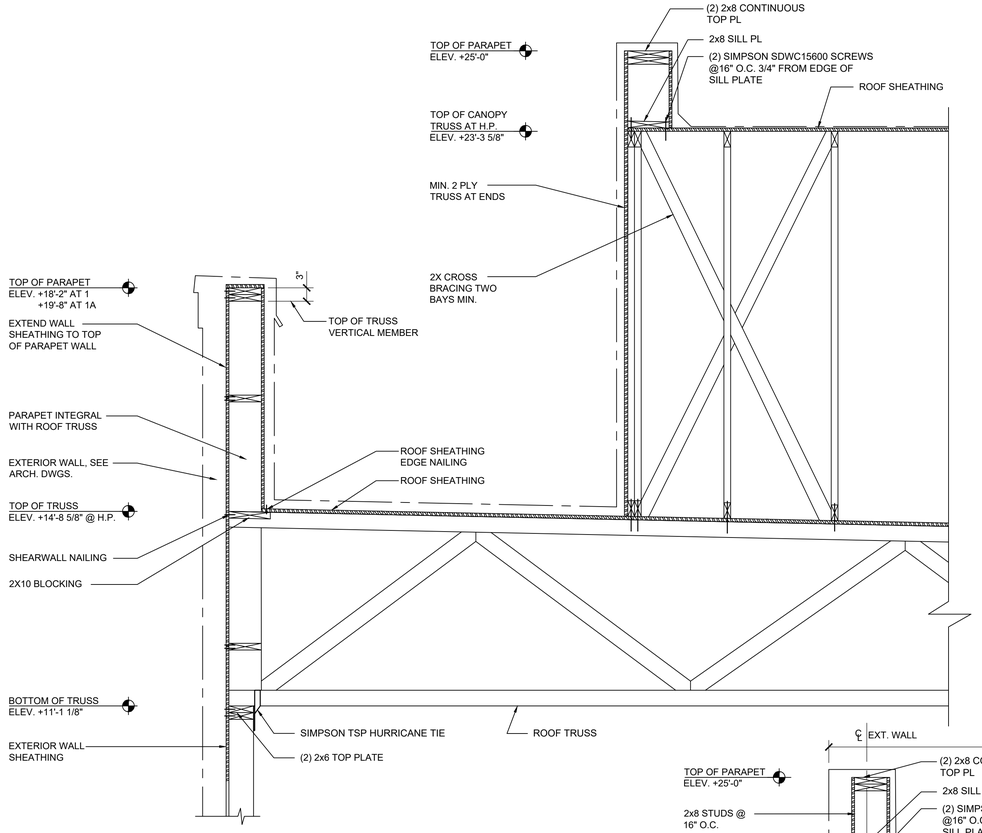
FRAMING SECTIONS

Drawing Number:

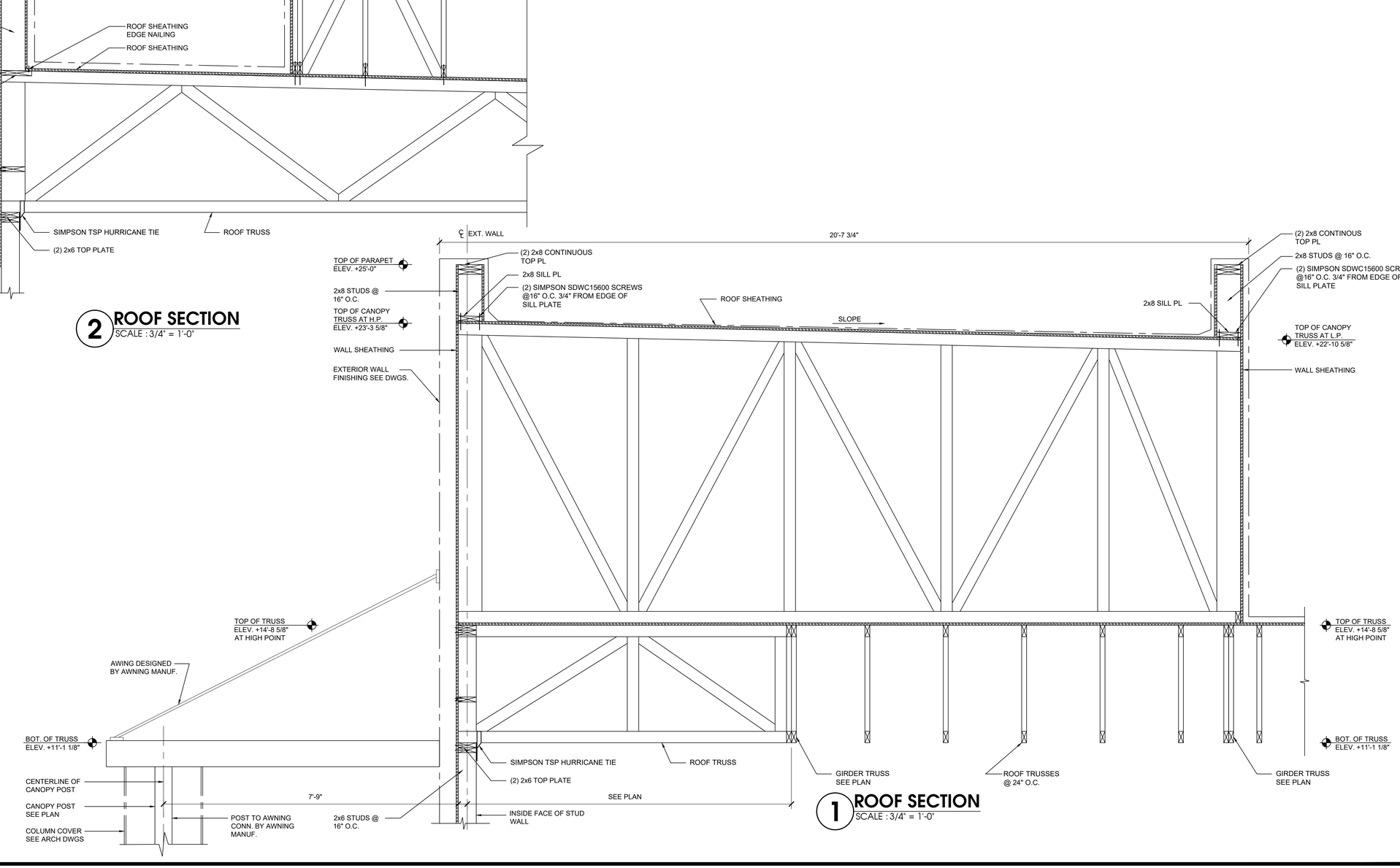
S-300

COMMONWEALTH OF VIRGINIA
SHANNON NICOLE CROWNOVER
Lic. No. 0402066228
PROFESSIONAL ENGINEER

2021.Q4.01 RETAIL PROTOTYPE



2 ROOF SECTION
SCALE : 3/4" = 1'-0"



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Drawn By: R.E.	Approved By: SC

Drawing Name:

FRAMING SECTIONS

Drawing Number:

S-301

COMMONWEALTH OF VIRGINIA
SHANNON NICOLE CROWNOVER
Lic. No. 0402066228
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2021.Q4.01 RETAIL PROTOTYPE

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