PART	1 - GEN		REQUIREMENTS AND DESIGN CRITERIA		
	SPECI			N.	ALL DETAILS, SE SITUATIONS ELS
. 1			R TO PROJECT SPECIFICATIONS FOR DETAILED REQUIREMENTS FOR MATERIAL AND WORKMANSHIP.	Ο.	WORK NOT INCL ELSEWHERE ON
.2				Ρ.	CONTRACTOR S
		COOF	LEVATIONS AND DIMENSIONS SHOWN FOR NEW CONSTRUCTION ARE BASED ON THE ARCHITECTURAL DRAWINGS. DINATE ALL ELEVATIONS AND DIMENSIONS BEFORE PROCEEDING WITH CONSTRUCTION.	Q.	RESPONSIBILITY PROVIDE 4" CON
			LEVATIONS AND DIMENSIONS SHOWN FOR NEW CONSTRUCTION ARE BASED ON THE ORIGINAL CONSTRUCTION DRAWINGS HE EXISTING BUILDING. FIELD VERIFY ALL ELEVATIONS AND DIMENSIONS BEFORE PROCEEDING WITH CONSTRUCTION.		ON GRADE OR O FLOORS. PAD S
	C.		YPICAL SECTIONS, ELEVATIONS, AND DIMENSIONS OF THE EXISTING STRUCTURE ARE OBTAINED FROM THE ORIGINAL DESIGN INGS BY METALLIC BUILDING COMPANY, DATED DECEMBER 02, 2003. EXISTING DIMENSIONS ARE TO BE VERIFIED IN THE	R. S.	DO NOT SCALE E PIPES OF 2" DIAM
		FIELD	BY THE CONTRACTOR, AS APPROPRIATE, PRIOR TO FABRICATION OF MEMBERS.		HANGERS FOR C
.3			BUILDING CODES OLLOWING BUILDING CODES AND STANDARDS, INCLUDING ALL SPECIFICATIONS REFERENCED WITHIN, SHALL APPLY TO THE		SECONDARY ME LOCATED TO KE
			INTERNATIONAL BUILDING CODE - 2018", INTERNATIONAL CODE COUNCIL	Т.	THE WEB AND B
		2.	"MINIMUM DESIGN LOADS FOR BUILDING AND OTHER STRUCTURES", (ANSI/ASCE 7-16, 2016), AMERICAN SOCIETY OF CIVIL ENGINEERS.	U.	ALL CMU WALLS
	В.		IONAL CODES FOR MATERIALS SHALL BE FOUND IN THE APPROPRIATE SECTIONS THAT FOLLOW. SEE THOSE SECTIONS FOR	V.	LINTELS: ALL OF
			PPLICABLE CODES.		REQUIRED TO PI LINTELS. COORI
.4		FLOO	R LIVE LOADS:		
			LOBBIES/STAIRS/EXITS100 PSF1.6MECHANICAL FLOOR AREAS150 PSF (UNLESS NOTED ON PLAN)1.6	SHOF A.	P DRAWINGS SHOP DRAWINGS
		3.	PARTITION LOADS 20 PSF		THE CONTRACTO
	В.	ROOF 1.	LIVE LOADS: MINIMUM LIVE LOAD: 20 PSF (USE SNOW LOAD IF GREATER)	В.	AND DESIGN OF
		2. 3.	ROOF TRUSS BOTTOM CHORD =15 PSFWIND UPLIFT=15 PSF	С.	REVIEW. SHOP DRAWINGS
	C.	ROOF	SNOW LOAD - PLUS DRIFTING AND SLIDING WHERE APPLICABLE	-	OF ELECTRONIC
	0.	11001	a. PG = 30 PSF b. PF = 24.5 PSF	D.	AT THE TIME OF OR OMISSIONS F
			c. CE = 1.0	E.	THE GENERAL C MAKE ALL CORR
			d. $I = 1.0$ e. $CT = 1.0$		WE CERT QUANTITIES ARE
	D.	DEAD	LOADS - ALL PERMANENT STATIONARY CONSTRUCTION.	F.	SIGNED ALLOW 15 BUSIN
	E.	WIND	LOAD PARAMETERS	G.	CONTRACTOR'S SHOP DRAWINGS
		1. 2.	BASIC WIND SPEED (3-SECOND GUST), V = 111 MPH WIND IMPORTANCE FACTOR, I = 1.0, AND OCCUPANCY CATEGORY = II		HAVE VERIFIED A
		3. 4.	EXPOSURE CATEGORY: C INTERNAL PRESSURE COEFFICIENT: GC(PI) = +/- 0.18	Н.	CONTRACTOR D
		5.	COMPONENTS AND CLADDING: a. ACTUAL PRESSURE(S) ON EVERY COMPONENT AND CLADDING ELEMENT SHALL BE DETERMINED BY THE		FOLLOWING ASS
			CONTRACTOR'S SPECIALTY PROFESSIONAL ENGINEER, LICENSED IN THE PROJECT'S JURISDICTION, RESPONSIBLE FOR THE STRUCTURAL DESIGN OF SUCH ELEMENT(S). THE HIGH PRESSURE CORNER ZONE DIMENSION SHALL BE		RESPONSIBILITY
			CALCULATED BASED ON THE OVERALL BUILDING DIMENSION AND SHALL APPLY TO ALL CORNERS (OUTSIDE AND INTERMEDIATE).		SUBMISSIONS SH
			b. PRESSURE VALUES LISTED BELOW ARE FOR REFERENCE ONLY:		1. CFMF AND a. DES
			INTERIOR ZONE: 20.0 PSF		COI L/60
			(ii) MAXIMUM OUTWARD PRESSURE ON VERTICAL FACE ON: END ZONE: -20.0 PSF INTERIOR ZONE: -20.0 PSF		DRA MAI
	F.				2. METAL ST a. DES
		1. 2.	SEISMIC IMPORTANCE FACTOR, I = 1.0, AND OCCUPANCY CATEGORY = II MAPPED SPECTRAL RESPONSE ACCELERATIONS		COI STF
			a. $S(S) = 0.126$ b. $S(1) = 0.045$		DES SHA
		3. 4.	SITE CLASS: C SPECTRAL RESPONSE COEFFICIENTS		DES 3. TEMPORA
			a. S(DS) = 0.109 b. S(D1) = 0.045		4. STRUCTUR 5. SPECIAL S
		5. 6.	SEISMIC DESIGN CATEGORY = A BASIC SEISMIC-FORCE-RESISTING SYSTEM: MASONRY SHEAR WALLS (PORT. A); ORDINARY MOMENT FRAMES (PORT. B)		6. CONCRET
		7. 8.	DESIGN BASE SHEAR, V = 14 KIPS (PORT. A); V = 3.7 KIPS (PORT. B) SEISMIC RESPONSE COEFFICIENT, C(S) = 0.01	F.	
		9.	ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE	G.	BY THE ARCHITE CONTRACTOR SI
.5	GENE A.		CTIONS AND CLOSEOUT: OWNER OR CONTRACTOR ARRANGED 3RD PARTY INSPECTIONS/ SPECIAL INSPECTIONS SHALL		OPENINGS REQU
	7.	CONF	ORM TO LOCAL JURISDICTION REQUIREMENTS AND INSPECTION REQUIREMENTS SHOWN ON THESE DOCUMENTS. FINAL FICATION FROM THE ENGINEER OF RECORD, IF REQUIRED BY THE AUTHORITY HAVING JURISDICTION OR OWNER, WILL BE		
		PROV	IDED ONCE TEC RECEIVES A THIRD PARTY INSPECTION REPORT CONFIRMING THE STRUCTURE WAS INSTALLED PER THE		
		CONF	RACT DOCUMENTS. TEC DOES NOT PROVIDE THIRD PARTY INSPECTIONS, AND OUR SITE OBSERVATIONS ARE FOR GENERAL ORMANCE, NOT INSPECTION.		
		DRAW	THORIZED REPRODUCTION OF ANY PORTION OF THE STRUCTURAL CONTRACT DRAWINGS FOR RE-SUBMITTAL AS SHOP INGS IS PROHIBITED. SHOP DRAWINGS PRODUCED IN SUCH A MANNER WILL BE REJECTED AND RETURNED.		
		EXCE	RT OF THE BUILDING SHALL BE USED AS A STAGING AREAS RESULTING IN A LOAD(UNDER THE LIMITED LOADED AREA) THAT EDS THE DESIGN LOADS.		
	D.	THE S	E DRAWINGS REPRESENT THE COMPLETED PROJECT WHICH HAS BEEN DESIGNED FOR THE WEIGHTS OF MATERIALS, FOR UPERIMPOSED LOADS INDICATED IN THE DESIGN LOAD CRITERIA ABOVE, AND FOR LOADS INDICATED ON THE DRAWINGS. IT		
			E CONTRACTOR'S RESPONSIBILITY TO DETERMINE ALLOWABLE CONSTRUCTION LOADS AND TO PROVIDE PROPER DESIGN CONSTRUCTION OF FALSE WORK, STAGING, BRACING, SHEETING AND SHORING, ETC.		
	E.		OPING AND IMPLEMENTING JOB SITE SAFETY AND CONSTRUCTION PROCEDURES ARE THE SOLE RESPONSIBILITY OF THE RACTOR.		
			ANGE IN SIZE, DIMENSION, OR POSITION OF STRUCTURAL ELEMENTS SHALL BE MADE, NOR SHALL ANY OPENINGS OR <u>PAF</u> /ES BE PERMITTED THROUGH ANY STRUCTURAL ELEMENT, WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER OF	<u> </u>	STING CONDITIONS
	DG S-001X_GENERAL NOTES.dwg	RECO	RD, UNLESS DETAILED AND SPECIFICALLY NOTED AND APPROVED ON THE SHOP DRAWINGS. PROVIDE SEPARATE SHOP 2.1 INGS INDICATED ALL PENETRATIONS THROUGH STRUCTURAL ELEMENTS FOR APPROVAL, PRIOR TO THE SUBMISSION OF THE	GENE A.	ERAL REQUIREMEN EXISTING BUILDI
		SHOP	DRAWINGS FOR THE AFFECTED STRUCTURAL ELEMENTS. OSTS OF INVESTIGATION AND REDESIGN, DUE TO THE CONTRACTOR MIS-LOCATION OF STRUCTURAL ELEMENTS OR OTHER		ON THE ARCHITE (DIMENSIONS, EL
	HE-PDG S-00	LACK	OF CONFORMANCE WITH THE PROJECT DOCUMENTS, SHALL BE AT THE CONTRACTOR'S EXPENSE. RACTOR SHALL REFER TO ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, DRAWINGS FOR SIZE AND LOCATION OF	В.	NEW FRAMED FL
	Winche	OPEN	INGS, SLEEVES, CONCRETE HOUSEKEEPING PADS, INSERTS AND DEPRESSIONS.	C.	OF THE STRUCT
	0	FIREF	RCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR DETAILED INFORMATION REGARDING FINISHES, WATERPROOFING, ROOFING, ETC. RCHITECTURAL DRAWINGS FOR LOCATIONS OF NON LOAD REARING MASONRY AND DRYMALL DARTITIONS AND PROVIDE SUR		AT THE SAME EL
		CONN	RCHITECTURAL DRAWINGS FOR LOCATIONS OF NON-LOAD BEARING MASONRY AND DRYWALL PARTITIONS AND PROVIDE SLIP ECTIONS THAT ALLOW VERTICAL MOVEMENT AT THE HEADS OF ALL SUCH PARTITIONS. UNLESS SHOWN ON THE DRAWINGS,	D.	INASMUCH AS TH MADE REGARDIN
	i\CAD\SHEE	LOAD	ONNECTIONS SHALL BE DESIGNED TO LATERALLY BRACE THE TOPS OF THE WALLS FOR THE CODE REQUIRED LATERAL PROVIDE COMPRESSIBLE FIRE-SAFING AT THE TOP OF RATED WALLS AS SPECIFIED BY THE ARCHITECTURAL DRAWINGS.		EXPENDING GRE
	hester-PDG		ULT ARCHITECTURAL , MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR LOCATIONS AND DIMENSIONS OF CHASES, TS, OPENINGS, SLEEVES, DRIPS, REVEALS, FINISHES, DEPRESSIONS, DOORS AND OTHER SUCH PROJECT REQUIREMENTS		THE OWNER WIL ALL CLAIMS, DAM
	Honda Winc	NOT S	HOWN ON THE STRUCTURAL DRAWINGS. ANY SUCH ITEMS SHOWN ON STRUCTURAL DRAWINGS ARE INDICATED FOR MATION ONLY. APPEARANCE OF SAME ON STRUCTURAL DRAWINGS IS NOT MEANT TO CONVEY ACTUAL LOCATION OR		
	Ÿ,	EXTE	IT OF WORK. IDE ANY ALTERATIONS AND/OR ADDITIONAL COMPONENTS NEEDED TO ACCOMMODATE THE INSTALLATION OF EQUIPMENT OF		
	ojects\23-0	ANY N	IATURE. COORDINATE SUCH WORK WITH THE EQUIPMENT SUPPLIER. INCORPORATE SUCH REFINEMENTS ON THE SHOP INGS, AND OBTAIN THE EQUIPMENT SUPPLIER'S APPROVAL (CLEARLY DISPLAYED ON THE DRAWINGS) PRIOR TO SUBMITTING		
	ts, PC\Pr		HOP DRAWINGS TO THE ARCHITECT AND ENGINEER FOR APPROVAL.		

M. IN CASE OF CONFLICT BETWEEN THE GENERAL NOTES. THE DETAILS AND SPECIFICATIONS. THE MOST RIGID SHALL GOVERN.

PART 3 - CONCRETE CTIONS, AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR 3.1 STANDARD SPECIFICATIONS AND REFERENCE STANDARDS SEWHERE UNLESS OTHERWISE NOTED. LUDED ON THE DRAWINGS BUT IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING LOCATIONS 1 THROUGH 5", AMERICAN CONCRETE INSTITUTE."MANUAL OF STANDARD STITUTE. THE DRAWINGS SHALL BE REPEATED. HALL TAKE ALL NECESSARY PRECAUTIONS TO PROTECT EXISTING AND NEW UTILITIES AND SHALL ASSUME FULL IRESSED CONCRETE", PRE-STRESSED CONCRETE INSTITUTE 'FOR ANY DAMAGE DURING CONSTRUCTION. G INSTITUTE. ICRETE PADS REINFORCED WITH #3 REBAR AT 12" E.W. AT MID DEPTH AT ALL EQUIPMENT SUPPORTED ON SLABS S WITH PRE-STRESSED UNBONDED TENDONS." ACI 423.3: ON FRAMED FLOORS UNLESS NOTE OTHERWISE. USE LIGHT WEIGHT CONCRETE FOR ALL PADS ON FRAMED SSED CONCRETE," PRE-STRESSED CONCRETE INSTITUTE HALL EXTEND 6" MINIMUM ON ALL SIDES OF THE EQUIPMENT. SPECIFICATIONS OF THE AMERICAN CONCRETE INSTITUTE DRAWINGS. R BUILDINGS METER OR LESS AND AIR DUCTS MAY BE SUSPENDED DIRECTLY FROM COMPOSITE DECK SLAB, WHERE APPLICABLE. ALL 3 CONSTRUCTION THER MECHANICAL PIPING AND EQUIPMENT SHALL BE CONNECTED TO THE STEEL BEAMS ONLY. ALL PIPE GROUPS PORTING AND PLACING CONCRETE ORTED ON TRAPEZES WHICH SHALL BE SUSPENDED FROM STEEL BEAMS OR JOISTS. CONTRACTOR MAY PROVIDE MBERS SPANNING BETWEEN STRUCTURAL BEAMS AS NEEDED. UNLESS NOTED OTHERWISE, HANGERS SHALL BE EP THE EQUIVALENT UNIFORM LOAD UNDER 10 PSF. OTTOM FLANGE OF STEEL BEAMS SHALL NOT BE USED FOR THE LATERAL SUPPORT OF CLADDING SYSTEMS UNLESS A NOT OTHERWISE SPECIFIED IDED AT THE POINT OF BRACING. THE SLOPE OF THE KICKER SHALL NOT BE STEEPER THAN 2 HORIZ TO 1 VERT. ON ELEVATED FRAMED FLOORS ARE INDICATED ON THE STRUCTURAL DRAWINGS. NO CMU WALLS ON ELEVATED SHOULD BE ADDED OR RELOCATED WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD. 3.2 CONCRETE MIX PROPERTIES: PENINGS IN WALLS AND PARTITIONS ARE TO BE PROVIDED WITH LINTELS. CONTRACTOR TO SHORE ALL LINTELS AS A. ELEMENT (NORMAL WEIGHT UNO) 28-DAY STRENGTH W/C MAX(b) REVENT ROTATION DURING CONSTRUCTION AND SHALL PAY PARTICULAR ATTENTION TO ECCENTRICALLY LOADED AIR CONTENT(a) DINATE SIZE, TYPE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. FOOTINGS 3,000 PSI 0.55 6% FOUNDATION WALLS AND PIERS: 4,000 PSI 0.50 6% 3,000 PSI 0.50 SLAB ON GRADE (INTERIOR) 3½% SLAB ON GRADE (EXTERIOR) 4,500 PSI 0.45 6% LESS THAN 3% 4,000 PSI 0.45 SLABS ON METAL DECK (LIGHTWEIGHT): S FOR ALL STRUCTURAL ELEMENTS SHOWN ON THE CONTRACT DOCUMENTS ARE REQUIRED TO BE SUBMITTED BY OR AND REVIEWED BY THE STRUCTURAL ENGINEER. IF A CONTRACTOR OR OWNER FAILS TO SUBMIT THE SHOP AIR CONTENT OF TROWEL FINISHED FLOORS SHALL NOT EXCEED 3% а. ANTINO ENGINEERING CONSULTANTS (TEC) WILL NOT BE RESPONSIBLE FOR THE STRUCTURAL CERTIFICATION PUMP MIXES: MAXIMUM WATER/CEMENT (W/C) RATIO MUST BE MAINTAINED. IF ADDITIONAL WORKABILITY IS THE PROJECT. REQUIRED FOR PUMPED PLACEMENT, THE HIGH OR MID-RANGE WATER REDUCERS SHALL BE USED IN LIEU OF S FOR HANGER LAYOUT ABOVE MECHANICAL ROOMS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR ADDITIONAL WATER. ELEVATED DECKS THAT ARE EXPOSED TO FREEZING TEMPERATURES, I.E. PLAZA DECKS, THAT DO NOT RECEIVE A C. S SUBMITTED FOR STRUCTURAL REVIEW SHALL CONSIST OF ELECTRONIC DRAWINGS. ONLY ONE MARKED UP SET TROWEL FINISH SHALL HAVE AN AIR CONTENT OF 6% DRAWINGS WITH THE STRUCTURAL ENGINEER'S COMMENTS WILL BE RETURNED TO THE CONTRACTOR. MAXIMUM SLUMP WITHOUT ADMIXTURES = 4" SHOP DRAWINGS SUBMISSION, THE CONTRACTOR SHALL INFORM THE ENGINEER IN WRITING OF ANY DEVIATIONS ROM THE CONTRACT DRAWINGS. ASTM C150, TYPE I OR III. USE TYPE II IN MARINE OR SUBMERGED ENVIRONMENTS. PORTLAND CEMENT: ONTRACTOR / CONSTRUCTION MANAGER SHALL REVIEW ALL SHOP DRAWINGS BEFORE SUBMITTING TO ENGINEER CEMENT SUBSTITUTES: ASTM C595, TYPE LS (LIMIT TO 50% MAX OF CEMENTITIOUS CONTENT BY WEIGHT) ECTIONS AS THEY DEEM NECESSARY AND SHALL CERTIFY ON EACH DRAWINGS AS FOLLOWS: AGGREGATES / DENSITY: MAXIMUM COARSE AGGREGATE SIZE = 3/4" TYPICAL, 1/2" MAXIMUM AT SLABS 3" OR LESS D. IFY THAT THE CONTRACT DOCUMENT REQUIREMENTS HAVE BEEN MET AND ALL DIMENSIONS, CONDITIONS, AND ASTM C33 / 145 PCF - NORMAL WEIGHT VERIFIED AS SHOWN AND/OR AS CORRECTED ON THIS DRAWINGS." ASTM C330 / 115 PCF - STRUCTURAL LIGHTWEIGHT ... (FOR CONTRACTOR)... AIR-ENTRAINMENT: ASTM C260 (ALL CONCRETE EXPOSED TO WEATHER AND WITHIN 4'-0" OF FINISHED GRADE). NESS DAYS FOR STRUCTURAL REVIEW OF SHOP DRAWINGS. THIS TIME SHOULD BE ALLOTTED IN THE SHOP DRAWINGS: CONCRETE MIX DESIGNS SHALL BE MADE BY AN APPROVED LABORATORY FOR ALL CONCRETE AND SHALL BE SCHEDULE. SUBMITTED TO THE ARCHITECT AND ENGINEER FOR APPROVAL BEFORE USE. THE MIX MUST CLEAR NOTE WHERE THE CONCRETE S SHALL BEAR THE CONTRACTOR'S STAMP OF APPROVAL WHICH SHALL CONSTITUTE CERTIFICATION THAT THEY IS INTENDED TO BE USED. ALL FIELD MEASUREMENTS, CONSTRUCTION CRITERIA, MATERIALS AND SIMILAR DATA AND HAVE CHECKED EACH G. CALCIUM CHLORIDE SHALL NOT BE PERMITTED IN CONCRETE IN ANY FORM. COMPLETENESS, COORDINATION AND COMPLIANCE WITH THE CONTRACT DOCUMENTS. ESIGNED COMPONENTS: THE CONTRACTOR SHALL SUBMIT FOR REVIEW, SIGNED AND SEALED DRAWINGS AND 3.3 BASE PLATE GROUT: 8,000 PSI 28-DAY COMPRESSIVE STRENGTH PREPARED BY A SPECIALTY STRUCTURAL ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION FOR THE EMBLIES. THIS REVIEW SHALL BE FOR GENERAL CONFORMANCE WITH THE PROJECT'S PARAMETERS AS 3.4 STEEL REINFORCEMENT: HE DRAWINGS, SPECIFICATIONS AND GENERAL NOTES. THE DESIGN OF THESE ASSEMBLIES IS THE DEFORMED REINFORCING BARS: ASTM A615 GRADE 60 OF THE CONTRACTOR'S ENGINEER WHO HAS SIGNED AND SEALED THESE DRAWINGS AND CALCULATIONS. THESE WELDABLE DEFORMED REINF. BARS: ASTM A706 OR APPROVED EQUAL HALL BE MADE AVAILABLE IN CONJUNCTION WITH OR PRIOR TO THE SHOP DRAWING FOR THE PRIMARY BUILDING WELDED WIRE REINFORCEMENT (WWR): ASTM A497 OR A185 (FLAT SHEETS ONLY) AT SUPPORT THESE ASSEMBLIES.) CURTAIN WALL SYSTEMS AND RELATED CONNECTIONS: 3.5 CONCRETE COVER: SIGNS SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY APPLICABLE BUILDING DES. BACK-UP SYSTEMS AND CURTAIN WALLS SHALL BE DESIGNED FOR A MAXIMUM LATERAL DEFLECTION OF MILD REINFORCED CONCRETE 00 OF THE SPAN IN INCHES OR $\frac{3}{8}$ " WHICHEVER IS LESS FOR THE APPLICABLE DESIGN WIND LOAD. THE SUBMITTED CONCRETE CAST AGAINST AND PERMANE WINGS AND CALCULATIONS SHALL CLEARLY SHOW THE LOAD PATH AND THE REACTIONS AS APPLIED TO THE CONCRETE EXPOSED TO EARTH OR WEAT IN BUILDING STRUCTURE. AIRS AND METAL RAILINGS: SIGNS SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY APPLICABLE BUILDING CONCRETE NOT EXPOSED TO WEATHER O DES. WHERE HEADERS OR OTHER TYPES OF STRUCTURAL MEMBERS HAVE BEEN DESIGNATED BY THE SLABS, WALLS AND JOISTS: RUCTURAL ENGINEER OF RECORD TO SUPPORT STAIRS, THE CONNECTIONS FROM THE STAIRS SHALL BE BEAMS AND COLUMNS: SIGNED SO THAT NO ECCENTRIC OR TORSIONAL FORCES ARE INDUCED INTO THESE MEMBERS. THE CONTRACTOR ALL BE RESPONSIBLE FOR FURNISHING AND INSTALLING EMBEDS AND HARDWARE AS REQUIRED BY THE STAIR SIGN. ARY CONDITIONS SUCH AS NEEDLING, SHORING AND BRACED EXCAVATION. 3.6 GENERAL REQUIREMENTS: RAL STEEL CONNECTIONS. REINFORCEMENT AT OPENINGS: UNLESS DETAILED OTHERWISE, PROVIDE 2 - #6 AT EACH SIDE OF ALL OPENINGS IN WALLS AND STEEL JOISTS. SLABS AND EXTEND 2 FT-6 IN. BEYOND THE OPENING OR AS DETAILED, EXCEPT VERTICAL BARS AT SIDES OF OPENINGS IN WALLS E FORMWORK. ARE TO EXTEND FROM FLOOR TO FLOOR. BARS MAY BE MOVED ASIDE AT OPENINGS OR SLEEVES, BUT DO NOT CUT OR OMIT. PROVIDE (2) #4 X 4'-0" AT SLAB MID DEPTH AT ALL RE-ENTRANT CORNERS OF FLOOR SLAB (BOTH ELEVATED AND SLAB ON GRADE). HALL FURNISH DIMENSIONED SHOP DRAWINGS AT ALL LEVELS LOCATING FLOOR AND ROOF EDGES FOR REVIEW C. MINIMUM REINFORCEMENT: REINFORCE ALL WALLS WITH AT LEAST #4 @ 12 IN. EACH WAY EACH FACE AND 2 - #6 EACH EDGE, ECT AND STRUCTURAL ENGINEER. UNLESS DETAILED OTHERWISE. HALL FURNISH DIMENSIONED SHOP DRAWINGS AT ALL LEVELS SHOWING LOCATIONS OF ALL SLEEVES AND D. CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL SO AS TO CAUSE SEGREGATION OF AGGREGATES. HOPPERS, JIRED BY ALL TRADES FOR REVIEW BY THE MEP. ARCHITECT AND STRUCTURAL ENGINEER. VERTICAL CHUTES, OR TRUNKS SHALL BE USED IN SUFFICIENT NUMBERS SO THAT THE FREE UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED SIX FEET AND TO ENSURE CONCRETE IS KEPT LEVEL AT ALL TIMES. EXISTING SURFACE TREATMENT: PRIOR TO PLACING FRESH CONCRETE AGAINST CONCRETE IN PLACE, THE CONTACT SURFACES

NTS

NG INFORMATION SHOWN IS BASED ON EXISTING BUILDING DRAWINGS, FIELD OBSERVATIONS, OR AS INDICATED ECTURAL DRAWINGS. THE CONTRACTOR SHALL VERIFY ALL EXISTING BUILDING INFORMATION SHOWN LEVATIONS, ETC.) AND NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES.

_OOR ELEVATIONS ARE TO MATCH EXISTING ADJACENT FLOOR ELEVATIONS UNLESS INDICATED OTHERWISE. EVATIONS OR THE LENGTH OF VERTICAL LOAD CARRYING ELEMENTS SHALL BE ADJUSTED WITH THE APPROVAL

URAL ENGINEER TO ACHIEVE MATCHING SLAB ELEVATIONS. OR DETAILED OTHERWISE ON THE PLANS, NEW FOUNDATIONS ADJACENT TO EXISTING FOUNDATIONS SHALL BEAR I FVATION

HE REMODELING AND/OR REHABILITATION OF AN EXISTING BUILDING REQUIRES THAT CERTAIN ASSUMPTIONS BE NG EXISTING CONDITIONS, AND BECAUSE SOME OF THESE ASSUMPTIONS CANNOT BE VERIFIED WITHOUT EAT SUMS OF ADDITIONAL MONEY, OR DESTROYING OTHERWISE ADEQUATE OR SERVICEABLE PORTIONS OF THE WNER AGREES THAT, EXCEPT FOR NEGLIGENCE ON THE PART OF TARANTINO ENGINEERING CONSULTANTS (TEC), L HOLD HARMLESS AND INDEMNIFY TARANTINO ENGINEERING CONSULTANTS (TEC) FOR AND AGAINST ANY AND MAGES, AWARDS, AND COSTS OF DEFENSE ARISING OUT OF DEFICIENCIES IN THE ORIGINAL BUILDING STRUCTURE.

STAN	DAR	D SPECIFICAT	IONS AND REFERENCE STANDA
A.	"AC	I MANUAL OF	CONCRETE PRACTICE - PARTS 1
	PR/	ACTICE", CON	CRETE REINFORCING STEEL INS
B.	"PC	I DESIGN HAN	IDBOOK - PRECAST AND PRE-ST
C.	"PC	ST-TENSIONII	NG MANUAL", POST-TENSIONING
D.	"RE	COMMENDAT	IONS FOR CONCRETE MEMBERS
E.	"PC	I STANDARD I	BUILDING CODE FOR PRE-STRES
F.	FOL	LOW THE LAT	FEST RECOMMENDATIONS AND S
	1.	ACI 301	STRUCTURAL CONCRETE FOR
	2.	ACI 302	CONCRETE FLOOR AND SLAB
	3.	ACI 304	MEASURING, MIXING, TRANSP
	4.	ACI 305	HOT WEATHER CONCRETING
	5.	ACI 306	COLD WEATHER CONCRETING
	6.	ACI 315	DETAILING REINFORCING STE
	7.	ACI 318	GENERAL DESIGN OF ITEMS N
	8.	ACI 347	FORMWORK

- F
- ALL KEYS SHALL BE 1.5" DEEP UNLESS NOTED OTHERWISE.
- CORES AND DRILLED FASTENERS:
- UNLESS AUTHORIZED IN WRITING BY THE STRUCTURAL ENGINEER. INSTALLED PER THE MANUFACTURER'S SPECIFICATIONS.
- SUBGRADE OR AS INDICATED ON THE PLANS.
- CONSPEC #1 BY CONSPEC MARKETING AND MANUFACTURING COMPANY OR APPROVED EQUAL. UNLESS NOTED OTHERWISE. SUBMIT FINISHED SCHEDULE TO ARCHITECT FOR APPROVAL.
- CHAMFER ALL EXPOSED CONCRETE CORNERS, 3/4 IN. X 3/4 IN. MINIMUM, UNLESS NOTED OTHERWISE ON THE ARCHITECTURAL J. DRAWINGS.
- CONSTRUCTION LOADS. THE CONCRETE FLOOR SURFACE SHALL BE LEVEL WITH MINIMUM THICKNESS SPECIFIED ON THE CONTRACT DOCUMENTS.
- STRUCTURES, UTILITIES, AND EXCAVATIONS AS REQUIRED FOR COMPLETION OF WORK.
- \cap
- HIGHER ELEVATION UNTIL THE INTENDED POUR IS COMPLETED.

NTLY EXPOSED TO EARTH: THER [.]	3 IN.
#6 BAR OR LARGER #5 BAR OR SMALLER OR IN CONTACT WITH GROUND:	2 IN. 1½ IN.
#11 BAR OR SMALLER TO TIES, STIRRUPS, OR SPIRALS	¾ IN. 1½ IN

SHALL BE CLEANED. ALL LAITANCE SHALL BE REMOVED, AND A CHEMICAL BONDING COMPOUND APPLIED. WHERE NOTED, ROUGHEN EXISTING CONCRETE SURFACES COMMON WITH NEW CONCRETE TO AMPLITUDE OF 1/4 INCH.

FORMWORK, SHORING AND RESHORING: SHALL BE DESIGNED AND SUBMITTED BY THE CONTRACTOR'S ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION WITH ALL SUBMISSIONS BEARING THE ENGINEER'S SEAL AND SIGNATURE.

INSERTS AND SLEEVES: CONTRACTOR SHALL FURNISH DIMENSIONED SHOP DRAWINGS AT ALL LEVELS SHOWING LOCATIONS OF ALL CAST-IN-PLACE SLEEVES, INSERTS AND OPENINGS REQUIRED BY ALL TRADES FOR REVIEW BY THE MEP, ARCHITECT AND STRUCTURAL ENGINEER, IN THAT ORDER. NO SLEEVE SHALL BE PLACED THROUGH ANY CONCRETE ELEMENT UNLESS SHOWN ON THE APPROVED SHOP DRAWINGS OR SPECIFICALLY AUTHORIZED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD.

1. DRILLED OR POWDER DRIVEN FASTENERS WILL BE PERMITTED WHEN PROVEN TO THE SATISFACTION OF THE STRUCTURAL ENGINEER THAT THE FASTENERS WILL NOT SPALL THE CONCRETE OR DAMAGE EXISTING REINFORCEMENT. 2. CORE DRILLING OF FOUNDATIONS, BEAMS, JOISTS, COLUMNS OR ANY POST-TENSIONED MEMBER SHALL NOT BE PERMITTED

WHEN INSTALLING EXPANSION OR ADHESIVE ANCHORS, THE CONTRACTOR SHALL TAKE MEASURES TO AVOID DRILLING OR CUTTING OF ANY EXISTING REINFORCING AND DESTRUCTION OF CONCRETE. ALL BOLTS AND ANCHORS SHALL BE

G. CONCRETE SLAB-ON-GRADE: SHALL BE 5" THICK, REINFORCED WITH 6 X 6 - W2.9 X 2.9 WWR AND PLACED ON A 6 MIL. VAPOR-RETARDER OVER A 4" MIN. LAYER OF CLEAN, WELL-GRADED GRAVEL OR CRUSHED STONE AND PROPERLY COMPACTED

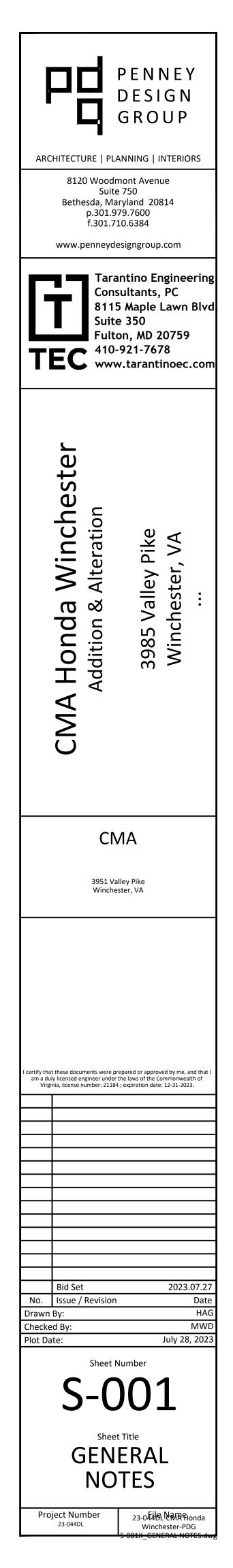
H. FLOOR SLABS SHALL BE FINISHED TO A MINIMUM FLATNESS F-NUMBER FF=20 AND A MINIMUM LEVELNESS F-NUMBER F1=17 IN ANY DIRECTION. ALL CONCRETE SHALL BE CURED WITH LIQUID SEALING COMPOUND CONFORMING TO ASTM C-309, TYPE 1 AND FEDERAL SPECIFICATION TT-C-00800 OR OTHER APPROVED METHOD WHICH IS COMPATIBLE WITH FLOORING ADHESIVES AND OTHER TREATMENTS. ALL CONCRETE LEFT EXPOSED AT THE COMPLETION OF THE PROJECT SHALL BE TREATED WITH A CLEAR, PENETRATING ACRYLIC BASED POLYMER CAPABLE OF PREVENTING INFILTRATION OF WATER BORNE CHLORIDES SUCH AS

ALL INTERIOR CONCRETE SHALL RECEIVE A STEEL TOWELED FINISH. EXTERIOR SLABS ON GRADE TO RECEIVE A BROOM FINISH,

K. WATERSTOPS: AS SPECIFIED ON THE ARCHITECTURAL DRAWINGS, PROVIDE CONTINUOUS WATERSTOPS AT ALL HORIZONTAL AND VERTICAL CONSTRUCTION JOINTS IN ALL BELOW GRADE FOUNDATION WALLS, ELEVATOR PITS AND OTHER PIT WALLS. CONCRETE QUANTITIES: THE CONCRETE SLABS SHALL BE FINISHED, WITHIN TOLERANCE AND FLOOR FLATNESS REQUIREMENTS, TO THE ELEVATIONS INDICATED ON THE DRAWINGS. CONTRACTOR SHALL PROVIDE AT THEIR COST, ADDITIONAL CONCRETE AS REQUIRED DUE TO FORMWORK AND FRAMING DEFLECTION TO ACHIEVE THIS FINISHED TOP OF SLAB ELEVATION. CONTRACTOR NOTE: THE STRUCTURAL FLOOR FRAMING SYSTEM UTILIZED ON THIS BUILDING IS CONCRETE POURED OVER METAL DECK SUPPORTED BY STEEL BEAMS. THIS FLOOR FRAMING HAS BEEN DESIGNED AS UNSHORED CONSTRUCTION AND WILL REQUIRE ADDITIONAL CONCRETE TO ACCOUNT FOR INITIAL BEAM DEFLECTIONS DUE TO THE WEIGHT OF THE WET CONCRETE AND

M. LOADS GREATER THAN THE DESIGN LIVE LOADS SHALL NOT BE PLACED ON THE STRUCTURE. A CONCRETE STRUCTURE MAY NOT SUPPORT ITS DESIGN LIVE LOAD UNTIL IT HAS REACHED ITS SPECIFIED STRENGTH. CONTRACTOR SHALL SUPPORT ADJACENT

IT IS NOT PERMISSIBLE TO DELAY THE APPLICATION OF CURING COMPOUND UNTIL THE MORNING AFTER THE CONCRETE IS CAST. CONCRETE CAST ON SLOPED SURFACES SHALL BEGIN AT THE LOWEST ELEVATION AND CONTINUE MONOLITHICALLY TOWARD THE



	A. CONDUITS IN CONCRETE SLABS SHALL BE SPACED SUCH THAT THE CENTER TO CENTER DISTANCE BETWEEN CONDUITS IS A MINIMUM OF THREE TIMES THE OUTSIDE DIAMETER OF THE LARGEST CONDUIT.	4.3	GENERA A. PI	AL: ROVIDE GALVANIZED STA
	B. CONDUITS IN CONCRETE SLAB HAVING OUTSIDE DIAMETER LARGER THAN ONE THIRD OF THE SLAB THICKNESS SHALL NOT BE		H	ORIZONTAL JOINT REINFO
	PERMITTED. CONDUITS THAT CROSS EACH OTHER WITHIN THE SLAB SHALL NOT CONSUME MORE THAN ONE THIRD OF THE SLAB THICKNESS AT THE POINT OF INTERSECTION. FOR ELEVATED SLABS WHICH ARE ON A DECK, THICKNESS SHALL BE DIVINED AS THE			I. O.C. AT ALL WALL CORN EINFORCEMENT IN FIRST
	CLEAR DIMENSION ABOVE THE RIBS. C. ALUMINUM CONDUITS WILL NOT BE PERMITTED IN CONCRETE ELEMENTS.			ROVIDE MASONRY ANCH BUTTING OR EMBEDDED
	D. LIGHTWEIGHT CONCRETE FILL OF SLAB DEPRESSIONS SHALL BE REINFORCED WITH FIBER REINFORCING.		C. Pl	ROVIDE CONTINUOUS BO
	E. ALL WELDING OF REINFORCING SHALL BE COMPLETED WITH E90XX ELECTRODES IN ACCORDANCE WITH AWS SPECIFICATION D1.4 (LATEST EDITION).			ROVIDE (2) #6 VERTICAL F T COLUMNS LOCATIONS,
3.7	SPLICING AND PLACEMENT OF REINFORCEMENT:			LONG THE HEIGHT OF TH
J.1	A. NO SPLICES OF REINFORCEMENT SHALL BE PERMITTED EXCEPT AS DETAILED OR AUTHORIZED BY THE STRUCTURAL ENGINEER.		W	YTHES AT 16" O.C. VERTI
	MAKE BARS CONTINUOUS AROUND CORNERS. WHEN PERMITTED, SPLICES SHALL BE MADE BY CONTACT TENSION LAP SPLICE, UNLESS NOTED OTHERWISE.			LL TOP CONNECTIONS OF OR INDEPENDENT VERTIC
	B. SPLICE WELDED WIRE REINFORCEMENT TWO FULL MESH LENGTHS AND WIRE TOGETHER.		H. Al	LL BEARING WALLS AND
	C. SPLICE BARS AS SHOWN ON DRAWINGS BUT NOT LESS THAN 50 BAR DIAMETERS FOR SLABS AND BEAM BOTTOM BARS, AND NOT LESS THAN 65 BAR DIAMETERS FOR WALLS AND BEAM TOP STEEL.			HE CMU UNLESS NOTED (LL CMU BEARING WALL C
	D. NO WELDING OF REINFORCING SHALL BE PERMITTED UNLESS SPECIFICALLY CALLED FOR OR APPROVED BY THE STRUCTURAL ENGINEER.			/EBS. LL PIERS AND PARTITION
	E. WELDED WIRE REINFORCING SHALL HAVE ENDS LAPPED ONE FULL PANEL AND SPLICE LACED WITH WIRE.		FL	LOOR AND ROOF CONSTR
	F. ANY MECHANICAL SPLICES USED, MUST BE "TENSION-COMPRESSION" TYPE AND SHALL COMPLY WITH ACI 318 UNLESS OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER. SHOP DRAWINGS SUBMITTED FOR THE ENGINEER'S APPROVAL MUST INDICATE THE			I MULTIPLE WYTHE WALLS REFABRICATED JOINT RE
	USE AND THE TYPE OF ANY MECHANICAL SPLICES USED. G. PROVIDE #4 CHAIR BARS, HIGH CHAIRS, TIES, CLIPS, SLAB BOLSTERS AND OTHER ACCESSORIES WHERE NOT SPECIFIED ON THE			I COMPOSITE WALLS WIT ROUT SOLID ALL VERTICA
	DRAWINGS IN ACCORDANCE WITH MANUAL OF STANDARD PRACTICE OR DETAILING REINFORCING CONCRETE STRUCTURES ACI		H	ORIZONTAL REINFORCING
	315 OR CRSI-WRSI MANUAL OF STANDARD PRACTICE. USE PLASTIC TIPS ON ALL CHAIRS PLACED ON THE SIDES OF CONCRETE FORMWORK.			OURSES OF WALLS. I GROUTED AND/OR REIN
	H. PROVIDE PLASTIC TIPPED BOLSTERS AND CHAIRS AT ALL LOCATIONS WHERE THE CONCRETE SURFACE IS IN CONTACT WITH THE BOLSTERS OR CHAIRS IS EXPOSED.		-	ONTINUOUS UNOBSTRUC AP SPLICES FOR DEFORM
			U. Al	LL WALL SECTIONS AND F
3.8	REINFORCEMENT SHOP DRAWINGS: A. SUBMIT FOR APPROVAL, COMPLETE BENDING AND PLACING DETAILS OF ALL REINFORCEMENT INCLUDING WELDED WIRE			ASONRY UNITS. ONTRACTOR SHALL PRO'
	REINFORCEMENT, INDICATING POSITION OF SPLICES. INCLUDE ACCESSORY DRAWINGS. ALL REINFORCING SHALL BE DETAILED,		IN	I PLACE.
	FABRICATED AND PLACED IN ACCORDANCE WITH ACI-315. B. UNAUTHORIZED REPRODUCTION OF ANY PORTION OF THE STRUCTURAL CONTRACT DRAWINGS FOR RE-SUBMITTAL AS SHOP		W	EE SPECIFICATIONS AND /ALLS AND PARTITIONS.
	DRAWINGS IS PROHIBITED. SHOP DRAWINGS PRODUCED IN SUCH A MANNER WILL BE REJECTED AND RETURNED.			HE CONTRACTOR SHALL INDOW FRAMES, ETC. AS
3.10			S	TRUCTURAL ENGINEER O
	A. LIGHTWEIGHT CONCRETE SHALL BE BATCHED AND MIXED AS SPECIFIED IN ACI 301 SECTION 7.0. PRESOAK AGGREGATES AS REQUIRED TO COMPLY WITH THIS SPECIFICATION AND TO FACILITATE CONCRETE PUMPING.			UBMIT SHOP DRAWINGS I TEEL INDICATING POSITIO
2 11	HOUSEKEEPING PADS AND CURBS:	1 1	LINTELS	
2.11	A. PADS AND CURBS MAY BE SHOWN ON PLAN IN CERTAIN INSTANCES FOR REFERENCE ONLY. SEE ARCHITECTURAL AND	4.4	A. A	LL OPENINGS IN NEW WA
	MECHANICAL DRAWINGS AND SPECIFICATIONS FOR LOCATIONS AND COORDINATE WITH EQUIPMENT MANUFACTURER'S REQUIREMENTS. USE SAME CONCRETE AS BASE SLAB, UNLESS DETAILED OTHERWISE. MAXIMUM PAD THICKNESS IS 6 INCHES.			TRUCTURAL STEEL. PRO OR LINTELS IN ALL EXTER
2.40			3	1/2" x 5/16" (LLV) ANGLE F
3.12	CONSTRUCTION JOINTS: A. CONSTRUCTION AND CONTROL JOINTS IN SLAB ON GRADE SHALL BE ARRANGED TO LIMIT MAXIMUM LENGTH BETWEEN JOINTS TO			OR EACH 4" OF WALL THIC Y THE ARCHITECT. PREC/
	15'-0" IN ANY DIRECTION. SUBMIT SHOP DRAWINGS INDICATING JOINT LAYOUT FOR ARCHITECT/ENGINEER APPROVAL. ALLOW A MINIMUM OF 48 HOURS TIME BETWEEN PLACEMENT OF ADJACENT SECTIONS.			PANS NOT EXCEEDING 6'- HAN 8'-0". ALL PRECAST (
	B. CONSTRUCTION JOINTS FOR MILD-REINFORCED CONCRETE SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF THE SPAN.		B. SI	EE ARCHITECTURAL, MEC
	PROPOSED CONSTRUCTION JOINT LOCATIONS SHALL BE SHOWN ON THE REINFORCING STEEL SHOP DRAWINGS. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULKHEADS AND HORIZONTAL KEYS, UNLESS INDICATED OTHERWISE.			TRUCTURAL ENGINEER F O OPENINGS SHALL BE PI
	FOUNDATIONS, PILE CAPS, DRILLED PIERS, SLABS, BEAMS, GIRDERS, AND JOISTS SHALL NOT HAVE JOINTS IN A HORIZONTAL PLANE		0	PENING BELOW THE LINT
	UNLESS DETAILED OTHERWISE. C. CONSTRUCTION JOINTS FOR SLABS ON METAL DECK SHALL BE LOCATED MIDWAY BETWEEN BEAMS THAT ARE PERPENDICULAR TO			ONTRACTOR SHALL SHOI ARTICULAR ATTENTION T
	THE DECK SPAN AND, IN THE MIDDLE THIRD OF THE SUPPORTING BEAM WHERE THE JOINT IS PARALLEL TO THE DECK SPAN. THE CONTRACTOR SHALL SUBMIT FOR APPROVAL A SHOP DRAWING INDICATING ALL PROPOSED JOINT LOCATIONS WITH ALL		E. Al	LL BEAM LINTELS LARGE
	ADDITIONAL REINFORCING STEEL TO BE PLACED IN THE SLAB. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL	4.4	-	TION AND TESTING:
	BULKHEADS AND HORIZONTAL KEYS. SLABS SHALL NOT HAVE JOINTS IN A HORIZONTAL PLANE, UNLESS DETAILED OTHERWISE. D. WHERE CONSTRUCTION JOINTS ARE PROVIDED THE REINFORCEMENT SHALL PASS CONTINUOUSLY THROUGH THE JOINT AND			HE OWNER SHALL ENGAG
	ADEQUATE SHEAR TRANSFER REINFORCEMENT SHALL BE PROVIDED.		B. TI	HE AGENCY SHALL CONT ROPORTIONING, MIXING A
3.13	CONCRETE SLAB ON GRADE CONSTRUCTION:			EINFORCING, CONNECTO
	A. THE CONCRETE SLABS ON GRADE FOR THIS PROJECT HAVE BEEN DESIGNED UTILIZING A MODULUS OF SUBGRADE REACTION "K" EQUAL TO 110 PCI FOR ALL AREAS. PLEASE NOTE THAT THE CONCRETE SLABS ON GRADE THROUGHOUT THIS PROJECT ARE NOT			UBMIT GROUT AND MORT OR APPROVAL.
	DESIGNED TO SUPPORT THE CRANES USED DURING THE ERECTION OF THE STRUCTURAL STEEL. IF THE CONTRACTOR ELECTS TO		D. O	BSERVE PREPARATION C
	PLACE THE CRANE ON THE CONCRETE SLAB ON GRADE, IT IS THE CONTRACTOR'S RESPONSIBILITY TO TAKE ALL NECESSARY PRECAUTIONS, INCLUDING THE TEMPORARY INSTALLATION OF WOOD CRIBBING ON THE SLAB, IN ORDER TO PREVENT CRACKS		-	ODE. HE CONTRACTOR SHALL
	FROM FORMING IN THE SLAB ON GRADE. ALL CRACKS WHICH FORM IN THE CONCRETE SLABS ON GRADE DUE TO THE CRANE BEING PLACED ON THE SLAB WILL BE REPLACED OR REPAIRED TO THE APPROVAL OF THE STRUCTURAL ENGINEER AND OWNER AT			3 DAYS. TESTS ARE TO B WO TESTS.
	THE CONTRACTOR'S EXPENSE.			NO 12313.
3.14	INSPECTION AND TESTING:			
	A. THE OWNER SHALL ENGAGE A TESTING AGENCY TO PROVIDE SERVICES AS INDICATED BELOW AND SUBMIT REPORTS.			
	 B. CAST-IN-PLACE CONCRETE: 1. THE AGENCY SHALL INSPECT THE FORMWORK AND REINFORCING STEEL PLACEMENT FOR COMPLIANCE WITH THE 	PART	T 5A - STRI	UCTURAL STEEL
	CONTRACT DOCUMENTS AND SHOP DRAWINGS. THE AGENCY SHALL MONITOR ALL STRUCTURAL CONCRETE PLACEMENT FOR CONFORMANCE WITH APPLICABLE ACI REQUIREMENTS.	5A.1	CODES:	
	2. SAMPLE FRESH CONCRETE IN ACCORDANCE WITH ASTM C172. MOLD TEST CYLINDERS IN ACCORDANCE WITH ASTM C31.			STEEL CONSTRUCTION M/ PECIFICATION FOR STRU
	 THE FOLLOWING NUMBER OF TEST CYLINDERS SHALL BE CAST FOR EACH DAY'S POUR OR EACH 50 CUBIC YARDS, WHICHEVER RESULTS IN MORE TEST CYLINDERS. 		C	ODE OF STANDARD PRAC
	a. FOR ELEVATED SLAB (TO INCLUDE BEAMS, JOISTS, GIRDERS): LAB CURED 2@7 DAYS, 2@28 DAYS			IANUAL OF STEEL CONST TEEL CONSTRUCTION.
	LAB CURED 2@7 DAYS, 2@28 DAYS FIELD CURED 2@7 DAYS, 2@28 DAYS		C. "S	STRUCTURAL WELDING C
	b. FOR FOOTINGS AND OTHER STRUCTURAL CONCRETE: LAB CURED 2@7 DAYS, 2@28 DAYS	5A.2		URAL SHAPES:
	c. THE AGENCY WILL MAKE ADDITIONAL TESTS OF IN-PLACE CONCRETE AT THE CONTRACTOR'S EXPENSE WHEN THE			/IDE FLANGE SHAPES: NGLES, PLATES AND CHA
	TEST RESULTS INDICATE SPECIFIED CONCRETE STRENGTHS HAVE NOT BEEN ATTAINED, AS DIRECTED BY THE STRUCTURAL ENGINEER.			TRUCTURAL PIPE: OUND HSS SHAPES:
				TRUCTURAL TUBING:
PART	<u>F4 - MASONRY</u>	5A.3	FASTEN	ERS, CONNECTORS:
4.1	CਊDES:	••		IGH STRENGTH BOLTS:
	A.ﷺ "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES, ACI 530 / ASCE 5 / TMS 402". B.ﷺ "SPECIFICATIONS FOR MASONRY STRUCTURES, ACI 530.1 / ASCE 6 / TMS 602".		B. Al	NCHOR RODS:
4.0				MOOTH OR THREADED RO EADED SHEAR STUDS:
4.2	MĀTERIALS:			ELDING ELECTRODES:
	1. HOLLOW AND SOLID ASTM C90, LIGHT WEIGHT, NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS = 1900 PSI.			F
	ALL UNITS TO BE TWO CORE TYPE. WEIGHT OF UNITS SHALL BE		F. E	XPANSION ANCHORS:
	30 PSF FOR 6" UNITS, 38 PSF FOR 8" UNITS 47 PSF FOR 10" UNITS AND 55 PSF FOR 12" UNITS WITH A			
	TOLERANCE OF 2 PSF.		G. Al	DHESIVE ANCHORS:
	B.ğ FACE BRICK ASTM C216 (CLAY OR SHALE), MINIMUM COMPRESSIVE STRENGTH ON NET AREA = 2000 PSI			
	C. MORTAR ASTM C270 - TYPE S. PROPORTIONS MIX BY VOLUME 1: 1½: 3 D. GROUT ASTM C476,	5A.4		G MATERIALS: EARING PADS: 60 DUR
	MINIMUM COMPRESSIVE STRENGTH ON NET AREA = 2000 PSI		"S	STANDARD SPECIFICATIO
	E. HORIZONTAL JOINT REINF. ASTM A82, 9-GAGE TRUSS-TYPE GALVANIZED F. COMP. STRENGTH OF MAS. F'M = 1900 PSI, UNIT STRENGTH METHOD OR PRISM TEST METHOD PER ACI 530 / ASCE 5			XPANSION JOINT BEARIN PPROVED EQUAL.
	G 🦉 WATER POTABLE	- • -		
	H. SAND ASTM C144, FINENESS MODULUS 2.0 TO 2.5 I. HYDRATED LIME ASTM C207, TYPE S	5A.5		RAWINGS: RECTION AND DETAIL DRA
	J. PORTLAND CEMENT ASTM C150, TYPE I OR II K. REINFORCING BARS ASTM A615, GRADE 60	5A F		ILL TEST RECORDS.
		54.5	A. SI	HOP FABRICATE TO GREA
				OLES AND CONNECTIONS LL STRUCTURAL STEEL P
	One Dive		B. Pf	RIOR TO STARTING FABR
			EI	TOTALLINT ON NEVIEW.

NIZED STANDARD WEIGHT HORIZONTAL JOINT REINFORCEMENT IN 8 IN. WALLS AND PARTITIONS AND EXTRA HEAVY INT REINFORCEMENT IN 12" WALLS AND PARTITIONS AT 16 IN. O.C. PROVIDE ONE PIECE PREFABRICATED UNITS AT 8 VALL CORNERS AND INTERSECTIONS. LAP REINFORCEMENT A MINIMUM OF TWO CELLS. PLACE JOINT NT IN FIRST AND SECOND BED JOINTS ABOVE AND BELOW OPENINGS.

NRY ANCHORS AT 16 IN. O.C. SET ON COURSING AND ATTACHED TO ALL BEAMS, COLUMNS, PARTITIONS, AND WALLS MBEDDED IN MASONRY.

NUOUS BOND BEAMS WITH (2) #4 HORIZONTAL REINFORCEMENT IN ALL MASONRY WALLS AT EACH FRAMING LEVEL. VERTICAL REINFORCEMENT FULL HEIGHT OF WALL AT ALL JAMB LOCATION UNLESS NOTED OTHERWISE. DCATIONS, ANCHOR MASONRY WALLS TO STEEL COLUMNS WITH FLEXIBLE WELD-ON TIES AT SPACING OF 16" MAX

GHT OF THE COLUMN. THE CMU WALLS SHALL BE TIED TOGETHER WITH LADDER TYPE HORIZ. JOINT REINFORCING ENGAGING BOTH O.C. VERTICAL REINFORCING UNLESS NOTED OTHERWISE.

CTIONS OF NON-LOAD BEARING MASONRY WALLS TO STRUCTURE MUST BE DETAILED TO PROVIDE A 1" SOFT JOINT ENT VERTICAL MOVEMENT OF THE PRIMARY STRUCTURAL MEMBER ABOVE (UNLESS NOTED OTHERWISE). ALLS AND ALL EXTERIOR MASONRY WALL SHALL BE REINFORCED WITH #5 AT 32" O.C. LOCATED IN THE CENTER OF

SS NOTED OTHERWISE. NG WALL CONSTRUCTION SHALL HAVE FULLY BEDDED MORTAR JOINTS. INCLUDING FACE SHELLS. HEADS AND

PARTITIONS SHALL BE BONDED OR ANCHORED TO ADJACENT MASONRY WALLS. PROVIDE TIES TO ADJACENT

OF CONSTRUCTION IN ACCORDANCE WITH DETAILS AND DRAWINGS. THE WALLS (CAVITY AND COMPOSITE WALLS) BOND THE WYTHES TOGETHER WITH RIGID METAL TIES OR

) JOINT REINFORCEMENT CONFORMING TO ACI 530 / ASCE 5 REQUIREMENTS. COMPLETELY FILL ALL COLLAR JOINTS VALLS WITH MORTAR OR GROUT. LL VERTICAL REINFORCED CELLS, CELLS BELOW GRADE, CELLS AT ANCHOR LOCATIONS, ALL BOND BEAMS WITH

INFORCING, ANY UN-REINFORCED CORES INDICATED TO RECEIVE GROUTING ON THE DRAWINGS, AND ALL TOP ALLS.

D/OR REINFORCED MASONRY WALLS, USE MASONRY UNITS WITH CORES THAT ALIGN VERTICALLY TO PROVIDE IOBSTRUCTED CELLS FOR GROUTING AND REINFORCING STEEL PLACEMENT. R DEFORMED REINFORCING BARS USED IN MASONRY CONSTRUCTION SHALL BE 48 BAR DIAMETERS.

ONS AND PIERS LESS THAN 4 FEET SQUARE IN CROSS-SECTIONAL AREA TO BE FULLY GROUTED OR OF 100% SOLID

HALL PROVIDE ADEQUATE BRACING AND SUPPORT OF ALL MASONRY WORK UNTIL PERMANENT CONSTRUCTION IS

FIONS AND DETAILS FOR GENERAL CONTROL JOINT REQUIREMENTS. JOINTS ARE TO BE CONSTRUCTED IN ALL RTITIONS.

OR SHALL VERIFY ALL OPENINGS BELOW LINTELS INDICATED ARE ADEQUATE TO ACCEPT DOOR FRAMES, LOUVERS, ES, ETC. AS SHOWN ON THE ARCHITECTURAL AND MECHANICAL DRAWINGS. NOTIFY THE ARCHITECT AND NGINEER OF ANY DISCREPANCIES PRIOR TO LINTEL INSTALLATION.

RAWINGS FOR DESIGNER'S APPROVAL INCLUDING COMPLETE BENDING AND PLACING DETAILS OF ALL REINFORCING NG POSITION OF SPLICES AND LOCATION OF REINFORCEMENT AND GROUT IN THE MASONRY.

N NEW WALLS AND PARTITIONS ARE TO BE PROVIDED WITH LINTELS. LINTELS SHALL BE STONE, CONCRETE, OR TEEL. PROVIDE 4" MINIMUM END BEARING FOR LINTELS IN NON BEARING PARTITIONS AND 8" MINIMUM END BEARING ALL EXTERIOR WALLS AND BEARING PARTITIONS. FOR ANY OPENING NOT SPECIFICALLY SHOWN, PROVIDE ONE 4" x /) ANGLE FOR EACH 4" OF WALL THICKNESS FOR SPANS NOT EXCEEDING 6'-0"; ONE 6" x 3 1/2" x 5/16" (LLV) ANGLE WALL THICKNESS FOR SPANS EXCEEDING 6'-0" BUT LESS THAN 8'-0" OR PRECAST CONCRETE LINTELS AS DIRECTED ECT. PRECAST CONCRETE LINTELS SHALL HAVE ONE #4 TOP AND BOTTOM FOR EACH 4" OF WALL THICKNESS FOR EEDING 6'-0"; ONE #5 TOP AND BOTTOM FOR EACH 4" OF WALL THICKNESS FOR SPANS EXCEEDING 6'-0" BUT LESS PRECAST CONCRETE LINTELS SHALL ALSO BE REINFORCED WITH #2 WIRE TIES AT 8"o/c.

URAL, MECHANICAL, ELECTRICAL AND STRUCTURAL DRAWINGS FOR LOCATIONS OF LINTELS. CONSULT IGINEER FOR LINTEL REQUIREMENTS FOR ALL NEW OPENINGS IN EXISTING WALLS.

HALL BE PLACED ABOVE ANY LINTEL WITHIN A HEIGHT LESS THAN OR EQUAL TO THE WIDTH OF THE CLEAR N THE LINTEL, UNLESS DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER. HALL SHORE ALL LINTELS AS REQUIRED TO PREVENT ROTATION DURING CONSTRUCTION AND SHALL PAY

FENTION TO ECCENTRICALLY LOADED LINTELS. LS LARGER THAN W8 BEAMS TO HAVE ADJUST MASONRY ANCHORS ON EACH FACE OF WEBS SPACED AT 16" o/c.

ALL ENGAGE A TESTING AGENCY TO PROVIDE SERVICES AS INDICATED BELOW AND SUBMIT REPORTS PER LEVEL 3 ANCE OF ACI 530.

ALL CONTINUOUSLY MONITOR THE FOLLOWING FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS:

A MIXING AND CONSISTENCY OF MORTAR AND GROUT, THE PLACEMENT OF MASONRY UNITS, GROUT. CONNECTORS, CONSTRUCTION OF MORTAR JOINTS, AND GROUT SPACE PRIOR TO GROUTING.

AND MORTAR MIX DESIGNS AND MASONRY UNIT AND MATERIAL CERTIFICATIONS TO THE STRUCTURAL ENGINEER

ARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS IN ACCORDANCE WITH THE MASONRY

OR SHALL PREPARE ONE SET OF PRISMS PER ASTM C-1314 FOR TESTING AT 7 DAYS AND ONE SET FOR TESTING AT ARE TO BE CONDUCTED BY THE AGENCY FOR EACH 5000 SQUARE FEET OF WALL INSTALLED, BUT NOT LESS THAN

UCTION MANUAL", THIRTEENTH EDITION, AMERICAN INSTITUTE OF STEEL CONSTRUCTION INC. 2016, (INCLUDING THE FOR STRUCTURAL STEEL BUILDINGS. SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS. AND THE ARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES).

EEL CONSTRUCTION, VOLUME II CONNECTIONS", ASD 14TH EDITION, (LRFD 3RD EDITION) AMERICAN INSTITUTE OF JCTION.

VELDING CODE - STEEL", AWS D1.1, AMERICAN WELDING SOCIETY. HAPES: ASTM A992 S AND CHANNELS: ASTM A36 ASTM A53, TYPE E, GRADE B, FY=35 KSI OR ASTM A501 PE: APES: ASTM A500, GRADE B, FY=42 KSI JBING: ASTM A500, GRADE B, FY=46 KSI TORS: BOLTS: ASTM A325-N (UNLESS DETAILED OTHERWISE) TENSION-CONTROL BOLTS ACCEPTABLE. ASTM F1554, GRADE 36 (UNLESS DETAILED OTHERWISE) READED ROD: ASTM A36 STUDS: ASTM A108, GRADE 1015 OR 1020

RODES: CONFORM TO AWS SPECIFICATIONS FOR ELECTRODES BASED ON WELDING PROCESS AND THE TYPE AND GRADE OF STEEL. E70XX ELECTRODES (MIN.) FOR FILLET WELDS. CHORS: HILTI KWIK BOLT 3 EXPANSION ANCHORS. INSTALL PER HILTI INSTALLATION RECOMMENDATIONS. UNLESS OTHERWISE NOTED EMBED BOLTS 10 BOLT DIAMETERS. IORS: HILTI HVA OR HIT SYSTEM AS SPECIFIED OR DETAILED

60 DUROMETER HARDNESS STRUCTURAL GRADE NEOPRENE MEETING SECTION 25, DIVISION 2, AASHTO CIFICATIONS FOR HIGHWAY BRIDGES", 2002 IT BEARING: FURON FLUOROGOLD SLIDE BEARING ASSEMBLY, CON-SERV CON-SLIDE BEARING ASSEMBLY OR

DETAIL DRAWINGS. RDS.

TE TO GREATEST EXTENT POSSIBLE BY WELDING INCLUDING BEAM STIFFENERS, COLUMN CAPS AND BASES, INECTIONS. SUBMIT COMPLETE SHOP DRAWINGS FROM FIELD DIMENSIONS FOR THE ARCHITECT'S APPROVAL OF AL STEEL PRIOR TO FABRICATION. TING FABRICATION, CERTIFIED COPIES OF THE MILL TEST REPORTS SHALL BE SUBMITTED TO THE STRUCTURAL

5A.6 ERECTION:

- PLATES IMMEDIATELY AFTER COLUMNS ARE PROVIDE BEARING PLATES AND WALL ANCH
- CONNECTING HARDWARE. SET ANCHOR RC C. DO NOT FIELD CUT OR FIELD MODIFY ANY ST
- FOR EACH SPECIFIC CASE.
- THE GENERAL CONTRACTOR SHALL NOTIFY WRITTEN APPROVAL BEFORE ANY FIELD CC WITHOUT THE APPROVAL OF THE STRUCTU
- F PERMANENT FRAMING AND FINAL CONNECT THE DESIGN OF TEMPORARY BRACING AND

5A.7 CONNECTIONS:

- A. ALL SHOP AND FIELD CONNECTIONS SHALL
- CLEARLY MARKED AS REQUIRED BY AISC SI PROVIDE ACCESS FOR INSPECTION OF ALL CONTRACTOR NOTE: ALTERNATE CONNECT С. APPROVAL IS GRANTED, ALL CONNECTIONS REDESIGN) SHALL BE DESIGNED BY THE FAI
- DRAWINGS SHALL BE SUBMITTED BEARING D. CONNECTIONS SHALL BE SELECTED FOR RE LESS THAN (2) ³/₄ IN. DIAMETER A325-N BOLT
- UNLESS DETAILED OTHERWISE, ALL A325 BC E. FEW IMPACTS OF AN IMPACT WRENCH OR T ENSURE THAT THE PLIES OF THE CONNECT
- ALL A325 BOLTS SUBJECT TO DIRECT TENSI WITH ONE OF THE FOLLOWING METHODS AS WRENCH TIGHTENING, OR DIRECT TENSION
- G. WHEN INSTALLING EXPANSION BOLTS OR AD EXISTING REINFORCING AND DESTRUCTION SPECIFICATIONS.
- WELDING ELECTRODES, WELDING PROCESS н AWS SPECIFICATIONS. ANY STRUCTURAL S ENGINEER.
- WELDERS SHALL HAVE CURRENT EVIDENCE EVIDENCE AT ANY TIME DURING THE PROJE

5A.8 STEEL FINISH:

- A. PAINT: SHOP PRIME ALL STEEL NOT ENCASE FINISH COAT REQUIREMENTS.
- ALL STEEL AT AND BELOW FINISHED GRADE COVER.
- C. ALL STRUCTURAL STEEL THAT IS LOCATED EXPOSED TO WEATHER, SHALL BE POWER-SPECIFICATIONS.
- D. ALL STRUCTURAL STEEL THAT IS SUBJECT BLAST CLEANED AND PAINTED WITH THREE SPECIFICATION NO. 13.01. A URETHANE TOF

5A.9 FRAMING:

- BEAMS ARE EQUALLY SPACED, UNLESS DIM
- CANTILEVER BEAMS ARE SAME SIZE AS BAC
- CAMBER INDICATED ON THESE DRAWINGS I C.
- CONCRETE SLABS THAT ARE PART OF COM OF ANY SUPERIMPOSED SPANDREL OR EDG

5A.10 INSPECTION AND TESTING:

- THE OWNER SHALL ENGAGE A TESTING AGE
- STRUCTURAL STEEL:
- VISUALLY INSPECT ALL FILLET WELDS THE AGENCY SHALL MONITOR THE INS
- TIGHTENING PROCEDURES.
- EACH FULL PENETRATION BUTT OR G 3
- ULTRASONIC METHOD. 10% OF ALL FIELD FILLET WELDS IN PF
- TEST ANY WELD WHICH VISUAL EXAM
- 6. WELDING INSPECTION AND TESTING

PART 5B - STEEL DECK

B

5B.1 CODES:

- "DESIGN MANUAL FLOOR DECKS AND ROOF DE
- SDI CODE OF RECOMMENDED PRACTICE AND S
- AISI SPECIFICATION FOR THE DESIGN OF COLD "STRUCTURAL WELDING CODE - STEEL", AWS D D.
- "STRUCTURAL WELDING CODE SHEET STEEL".

5B.2 MATERIALS:

- GALVANIZED METAL DECK
- В. PAINTED PHOSPHATED METAL FLOOR DECK

5B.3 GENERAL:

- A. DECK PROPERTIES ARE BASED ON PRODUCTS PROVIDED SECTION PROPERTIES ARE WITHIN S PROVIDE STEEL DECK WITH THE FOLLOWING M
- DECK TYPE 9/16" DEEP - 26 GAGE, UFS FORM DECK
- 2. 1¹/₂" DEEP 22 GAGE, TYPE B, ROOF DECK
- C. INSTALL IN ACCORDANCE WITH SDI SUGGESTE
- AT LEAST THREE SPANS, WITH LAPS TO BE PLA
- EXCEPT WHERE NOTED ON PLANS, DECK SUPP CLOSURE ANGLES AND PLATES WERE REQUIRE
- USE WELDING WASHERS FOR DECK MATERIAL COMPOSITE DECKS SHALL BE WELDED TO ALL
- FUSION DIAMETER) PLUG WELDS OR HEADED S 30 IN O.C. HEADED STUDS SHALL BE FIELD INST ROOF AND NON-COMPOSITE DECKS SHALL BE G.
- DIAMETER (EFFECTIVE FUSION DIAMETER) PLU MAXIMUM SPACING OF 12" O.C.
- 5B.4 POUR STOPS/DECK CLOSURE:
 - PROVIDE SHEET METAL POUR STOPS WITH THIC Α. PROVIDE CELL CLOSURE AS REQUIRED. SEE ARCHITECTURAL DRAWINGS FOR DECKS E C.
- PART 5C STEEL JOISTS
- 5C.1 CODES: "STANDARD SPECIFICATIONS FOR OPEN WEB S Α. "STANDARD SPECIFICATIONS FOR LONGSPAN S C. "STANDARD SPECIFICATION FOR JOIST GIRDER
- 5C.2 STEEL JOISTS:
- A. PROVIDE OPEN-WEB STEEL JOISTS CONFORMI DOCUMENTS. PROVIDE BOTTOM CHORD EXTEN
- 5C.3 UPLIFT:
- A. DESIGN AND ANCHOR ROOF JOISTS TO RESIST TO BRACE THE BOTTOM CHORD, INCLUDING TH
- 5C.4 BRIDGING:
 - Α. LOADS.

PROVIDE ANCHOR RODS, STEEL WEDGES, THREADED SCREWS OR SHIMS TO SUPPORT AND PLUMB ALL COLUMNS. GROUT SOLID UNDER BASE	I
PLATES IMMEDIATELY AFTER COLUMNS ARE PLUMB. PROVIDE BEARING PLATES AND WALL ANCHORS OR ANCHOR RODS FOR ALL BEAMS RESTING ON CONCRETE AND ALL OTHER NECESSARY	_
CONNECTING HARDWARE. SET ANCHOR RODS USING TEMPLATE. DO NOT FIELD CUT OR FIELD MODIFY ANY STRUCTURAL STEEL WITHOUT PRIOR WRITTEN APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER	PENNEY
FOR EACH SPECIFIC CASE. THE GENERAL CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY FABRICATION OR ERECTION ERRORS OR DEVIATIONS AND RECEIVE WRITTEN APPROVAL BEFORE ANY FIELD CORRECTIONS ARE MADE. GAS CUTTING TORCHES SHALL NOT BE USED TO CORRECT FABRICATION ERRORS	DESIGN
WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER. PERMANENT FRAMING AND FINAL CONNECTION DETAILS ARE SHOWN ON THE DRAWINGS. THE FABRICATOR AND ERECTOR ARE RESPONSIBLE FOR THE DESIGN OF TEMPORARY BRACING AND RECOMMENDED ERECTION PROCEDURES.	
NECTIONS:	ARCHITECTURE PLANNING INTERIORS
ALL SHOP AND FIELD CONNECTIONS SHALL BE MADE WITH HIGH STRENGTH BOLTS OR WELDS. ALL HIGH STRENGTH BOLTS AND NUTS SHALL BE CLEARLY MARKED AS REQUIRED BY AISC SPECIFICATION. CONNECTIONS MADE WITH UNMARKED BOLTS AND NUTS WILL BE REJECTED. PROVIDE ACCESS FOR INSPECTION OF ALL SHOP AND FIELD CONNECTIONS FOR PROPER MATERIAL AND WORKMANSHIP. CONTRACTOR NOTE: ALTERNATE CONNECTION DESIGNS SHALL ONLY BE ALLOWED WITH PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. IF SUCH APPROVAL IS GRANTED, ALL CONNECTIONS, SPLICES AND ERECTION PIECES NOT IN ACCORDANCE WITH THE CONTRACT DOCUMENTS (FABRICATOR'S	8120 Woodmont Avenue Suite 750 Bethesda, Maryland 20814 p.301.979.7600 f.301.710.6384 www.penneydesigngroup.com
REDESIGN) SHALL BE DESIGNED BY THE FABRICATOR'S ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION. CALCULATIONS AND SHOP DRAWINGS SHALL BE SUBMITTED BEARING THE ENGINEER'S SEAL AND SIGNATURE.	
CONNECTIONS SHALL BE SELECTED FOR REACTIONS AS SHOWN ON PLANS AND AS DETAILED AND SCHEDULED. NO CONNECTION SHALL CONSIST OF LESS THAN (2) ³ / ₄ IN. DIAMETER A325-N BOLTS OR WELDS DEVELOPING LESS THAN 10,000 POUNDS (UNFACTORED). MINIMUM WELD: ³ / ₁₆ IN. FILLET. UNLESS DETAILED OTHERWISE, ALL A325 BOLTS SHALL BE TIGHTENED TO THE 'SNUG TIGHT' CONDITION DEFINED AS THE TIGHTNESS ATTAINED BY A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH. THE SNUG TIGHT CONDITION MUST ENSURE THAT THE PLIES OF THE CONNECTED MATERIALS HAVE BEEN BROUGHT INTO SNUG CONTACT. ALL A325 BOLTS SUBJECT TO DIRECT TENSION OR DESIGNED AS SLIP CRITICAL (NOTED SC IN DETAILS) SHALL BE PRE-TENSIONED IN ACCORDANCE WITH ONE OF THE FOLLOWING METHODS AS DESCRIBED IN THE AISC "MANUAL OF STEEL CONSTRUCTION": TURN-OF-NUT TIGHTENING, CALIBRATED WRENCH TIGHTENING, OR DIRECT TENSION INDICATOR TIGHTENING.	Tarantino Engineering Consultants, PC 8115 Maple Lawn Blvd Suite 350 Fulton, MD 20759 410-921-7678 www.tarantinoec.com
WHEN INSTALLING EXPANSION BOLTS OR ADHESIVE ANCHORS, THE CONTRACTOR SHALL TAKE MEASURES TO AVOID DRILLING OR CUTTING OF ANY EXISTING REINFORCING AND DESTRUCTION OF CONCRETE. ALL BOLTS AND ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S SPECIFICATIONS.	
WELDING ELECTRODES, WELDING PROCESS, MINIMUM PREHEAT AND INTERPASS TEMPERATURES SHALL BE IN ACCORDANCE WITH THE AISC AND AWS SPECIFICATIONS. ANY STRUCTURAL STEEL DAMAGED IN WELDING IS TO BE REPLACED OR REINFORCED AS ACCEPTABLE TO THE STRUCTURAL ENGINEER.	
WELDERS SHALL HAVE CURRENT EVIDENCE OF PASSING THE APPROPRIATE AWS QUALIFICATION TESTS. THE ENGINEER MAY REQUEST SUCH EVIDENCE AT ANY TIME DURING THE PROJECT.	
EL FINISH:	te
PAINT: SHOP PRIME ALL STEEL NOT ENCASED IN CONCRETE OR NOT FIREPROOFED. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR FINISH COAT REQUIREMENTS.	est
ALL STEEL AT AND BELOW FINISHED GRADE OR FLOOR SLAB SHALL RECEIVE TWO (2) COATS OF BITUMINOUS PAINT OR 3 IN. MINIMUM CONCRETE COVER. ALL STRUCTURAL STEEL THAT IS LOCATED IN EXTERIOR UNHEATED SPACES AND WHICH IS EXPOSED FOR AESTHETICS, INCLUDING STEEL DIRECTLY	inch inch ration va VA
EXPOSED TO WEATHER, SHALL BE POWER-TOOLED CLEANED AND PAINTED OR GALVANIZED ACCORDING TO DETAILS AND ARCHITECT'S SPECIFICATIONS.	
ALL STRUCTURAL STEEL THAT IS SUBJECT TO WETTING WITH SALT-LADEN WATER OR OTHER MILD CHEMICAL ATTACK SHALL BE COMMERCIALLY BLAST CLEANED AND PAINTED WITH THREE COATS OF EPOXY PAINT IN ACCORDANCE WITH THE STEEL STRUCTURES PAINTING COUNCIL SYSTEM SPECIFICATION NO. 13.01. A URETHANE TOPCOAT SHALL BE PROVIDED FOR ALL STEEL EXPOSED TO VIEW.	کر گھ all all
IING: BEAMS ARE EQUALLY SPACED, UNLESS DIMENSIONED OTHERWISE ON PLAN. CANTILEVER BEAMS ARE SAME SIZE AS BACKSPAN, UNLESS NOTED OTHERWISE ON PLAN. CAMBER INDICATED ON THESE DRAWINGS IS THE REQUIRED CAMBER AT THE TIME OF ERECTION BEFORE PLACEMENT OF DECK. CONCRETE SLABS THAT ARE PART OF COMPOSITE FLOOR FRAMING SYSTEMS SHALL ACHIEVE 28-DAY DESIGN STRENGTH PRIOR TO THE APPLICATION OF ANY SUPERIMPOSED SPANDREL OR EDGE OF SLAB LOADS SUCH AS CURTAIN WALLS, MASONRY VENEERS, AND STAIRS.	Honda Addition a 3985 V Winch
ECTION AND TESTING: THE OWNER SHALL ENGAGE A TESTING AGENCY TO PROVIDE SERVICES AS INDICATED BELOW AND SUBMIT REPORTS.	AA
 STRUCTURAL STEEL: VISUALLY INSPECT ALL FILLET WELDS, BOLTED CONNECTIONS AND SHEAR STUDS. THE AGENCY SHALL MONITOR THE INSTALLATION OF BOLTS REQUIRING PRE-TENSIONING FOR CONFORMANCE WITH SPECIFIC PRE-CALIBRATED TIGHTENING PROCEDURES. EACH FULL PENETRATION BUTT OR GROOVE WELD AND FIFTY PERCENT OF PARTIAL PENETRATION WELDS SHALL BE TESTED BY THE ULTRASONIC METHOD. 	5
 4. 10% OF ALL FIELD FILLET WELDS IN PRIMARY CONNECTIONS AND MULTI-PASS WELDS SHALL BE TESTED BY THE MAGNETIC PARTICLE METHOD. 5. TEST ANY WELD WHICH VISUAL EXAMINATION INDICATES AN UNUSUAL CONDITION AND/OR POOR QUALITY. 6. WELDING INSPECTION AND TESTING PROCEDURES SHALL BE IN ACCORDANCE WITH THE AWS CODE. 	
EL DECK	CMA
	3951 Valley Pike
ESIGN MANUAL FLOOR DECKS AND ROOF DECKS", STEEL DECK INSTITUTE. DI CODE OF RECOMMENDED PRACTICE AND SPECIFICATIONS FOR COMPOSITE STEEL FLOOR DECK. SI SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS. TRUCTURAL WELDING CODE - STEEL", AWS D1.1, AMERICAN WELDING SOCIETY. TRUCTURAL WELDING CODE - SHEET STEEL", AWS D1.3, AMERICAN WELDING SOCIETY.	Winchester, VA
ALS: ALVANIZED METAL DECK ASTM A653 (G90) AINTED PHOSPHATED METAL FLOOR DECK ASTM A611	
L: ECK PROPERTIES ARE BASED ON PRODUCTS MANUFACTURED BY UNITED STEEL DECK, INC. (USD). DECKS BY OTHER MANUFACTURERS MAY BE SUPPLIED ROVIDED SECTION PROPERTIES ARE WITHIN 5% OF THOSE SPECIFIED AND IF APPROVED BY THE ARCHITECT AND STRUCTURAL ENGINEER. ROVIDE STEEL DECK WITH THE FOLLOWING MINIMUM SECTION PROPERTIES: DECK TYPE IP (IN^4) SP(IN^3) SN(IN^3)	
9/16" DEEP - 26 GAGE, UFS FORM DECK 0.015 0.046 0.047 1½" DEEP - 22 GAGE, TYPE B, ROOF DECK 0.170 0.190 0.200	I certify that these documents were prepared or approved by me, and that I am a duly licensed engineer under the laws of the Commonwealth of Virginia, license number: 21184 ; expiration date: 12-31-2023.
STALL IN ACCORDANCE WITH SDI SUGGESTED SPECIFICATIONS UNLESS NOTED OTHERWISE ON THE DRAWINGS. INDIVIDUAL SHEETS SHALL EXTEND OVER I LEAST THREE SPANS, WITH LAPS TO BE PLACED OVER SUPPORTS.	
(CEPT WHERE NOTED ON PLANS, DECK SUPPLIER SHALL PROVIDE ALL ADDITIONAL FRAMING TO SUPPORT DECK AT OPENINGS THROUGH DECK AND ALL OSURE ANGLES AND PLATES WERE REQUIRED TO RESULT IN A COMPLETE INSTALLATION.	
SE WELDING WASHERS FOR DECK MATERIAL 0.028 IN. THICK OR LESS AND WHERE RECOMMENDED BY THE DECK MANUFACTURER. OMPOSITE DECKS SHALL BE WELDED TO ALL SUPPORTS INCLUDING THE EDGE SUPPORT PARALLEL TO THE DECK SPAN WITH 5/8 IN. DIAMETER (EFFECTIVE	
JSION DIAMETER) PLUG WELDS OR HEADED STUDS WELDS AT 12 IN. O.C. FASTEN SIDE LAPS WITH 1½ IN. SEAM WELDS OR #10 SELF-TAPPING SCREWS AT IN O.C. HEADED STUDS SHALL BE FIELD INSTALLED BY WELDING THROUGH THE METAL DECK.	
DOF AND NON-COMPOSITE DECKS SHALL BE WELDED TO STEEL SUPPORTS, INCLUDING THE EDGE SUPPORT PARALLEL TO THE DECK SPAN WITH 5/8 IN. AMETER (EFFECTIVE FUSION DIAMETER) PLUG WELDS IN 36/4 PATTERNS. FASTEN SIDE LAPS WITH 1½" SEAM WELDS OR #10 SELF-TAPPING SCREWS AT A AXIMUM SPACING OF 12" O.C.	
DUR STOPS/DECK CLOSURE:	
ROVIDE SHEET METAL POUR STOPS WITH THICKNESS BASED ON SDI CRITERIA (SDI PUBLICATION #29); 14 GAUGE MIN. THICKNESS. ROVIDE CELL CLOSURE AS REQUIRED.	
EE ARCHITECTURAL DRAWINGS FOR DECKS EDGES AT ROOFING CONDITIONS.	Bid Set 2023.07.27 No. Issue / Revision Date
TEEL JOISTS	Drawn By: HAG
TANDARD SPECIFICATIONS FOR OPEN WEB STEEL, K-SERIES JOIST", STEEL JOIST INSTITUTE. TANDARD SPECIFICATIONS FOR LONGSPAN STEEL JOISTS, LH-SERIES AND DEEP LONGSPAN STEEL JOISTS, DLH-SERIES", STEEL JOIST INSTITUTE. TANDARD SPECIFICATION FOR JOIST GIRDERS", STEEL JOIST INSTITUTE.	Checked By: MWD Plot Date: July 28, 2023 Sheet Number
DISTS: ROVIDE OPEN-WEB STEEL JOISTS CONFORMING TO THE REQUIREMENTS OF THE SJI. FOR SIZE, TYPE, LENGTH AND SPACING SEE THE CONTRACT DCUMENTS. PROVIDE BOTTOM CHORD EXTENSIONS AT ALL COLUMNS AND WHERE SPECIFIED BY THE ARCHITECTURAL DRAWINGS.	S-002
ESIGN AND ANCHOR ROOF JOISTS TO RESIST A MINIMUM UPLIFT OF 15 PSF. JOIST MANUFACTURER SHALL PROVIDE ADDITIONAL BRIDGING AS NEEDED O BRACE THE BOTTOM CHORD, INCLUDING THE FIRST PANEL POINT FROM THE SUPPORT, TO RESIST ASSOCIATED COMPRESSION STRESSES.	Sheet Title GENERAL
G: DOVIDE RRIDCING AND CROSS RRACING CONFORMING TO THE REQUIREMENTS OF THE SUL JOIST SURDUER TO RROVIDE CONNECTIONS FOR CROSS	NOTES

PROVIDE BRIDGING AND CROSS-BRACING CONFORMING TO THE REQUIREMENTS OF THE SJI. JOIST SUPPLIER TO PROVIDE CONNECTIONS FOR CROSS BRACING. ALL BRIDGING, BRIDGING ANCHORS AND JOIST SEATS SHALL BE COMPLETELY INSTALLED PRIOR TO THE APPLICATION OF ANY CONSTRUCTION

Project Number

23-044DL

23-0446L MARHonda

Winchester-PDG

5C.5	CONCENTRATED LOADS: A. APPLY SUSPENDED OR ROOF TOP CONCENTRATED LOADS AT PANEL POINTS OR PROVIDE SUPPLEMENTAL FRAMING TO TRANSFER	PART	3 - FOUNDATIONS / EARTHWORK / C
	LOADS TO PANEL POINTS.	31.1	REFERENCE GEOTECHNICAL REPO A. FOUNDATION DESIGN IS IN A
5C.6	PAINT: A. SHOP PAINT JOISTS.		TIM STOWE, PE, DATED MAR B. FOUNDATIONS HAVE BEEN I PER THE GEOTECHNICAL RE
PART	5D - STRUCTURAL COLD FORMED METAL FRAMING (CFMF)		C. ALL FOUNDATIONS SHALL B THESE REQUIREMENTS WIT
	CODES: A. "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS", AMERICAN IRON AND		D. SEE THE SPECIFICATIONS A
	STEEL INSTITUTE. B. "STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS", AMERICAN IRON AND STEEL INSTITUTE.		THE BEARING MATERIAL, AN COMPACTION PROCEDURES E. IN THE ABSENCE OF A GEOT
5D.2	MATERIALS: A. STEEL SHEET: ASTM A 1003, STRUCTURAL GRADE, TYPE H, METALLIC COATED, OF GRADE AND COATING WEIGHT AS FOLLOWS:		F. REQUIREMENTS CONTAINED
	 GRADE: [ST33H] [ST50H] [AS REQUIRED BY STRUCTURAL PERFORMANCE] <insert grade="">.</insert> COATING: [G60, A60, AZ50, OR GF30] [G90 OR EQUIVALENT] <insert coating="" weight="">.</insert> 		G. REFER TO GEOTECHNICAL F H. IF UNSUITABLE BEARING MA
	B. STEEL SHEET FOR [VERTICAL DEFLECTION] [DRIFT] CLIPS: ASTM A 653, STRUCTURAL STEEL, ZINC COATED, OF GRADE AND COATING AS FOLLOWS:	31.2	FOUNDATION DESIGN PARAMETER A. SPREAD FOOTINGS:
	 GRADE: [50, CLASS 1 OR 2] [AS REQUIRED BY STRUCTURAL PERFORMANCE]. COATING: G90. 		1. BUILDING SPREAD AN COMPACTED ENGINEI
5D.3	ANCHORS, CLIPS, AND FASTENERS A. STEEL SHAPES AND CLIPS: ASTM A 36, ZINC COATED BY HOT-DIP PROCESS ACCORDING TO ASTM A 123.	31.3	
	B. ANCHOR BOLTS: ASTM F 1554, GRADE 55, THREADED CARBON-STEEL HEX-HEADED BOLTS AND CARBON-STEEL NUTS; AND FLAT, HARDENED-STEEL WASHERS; ZINC COATED BY HOT-DIP PROCESS ACCORDING TO ASTM A 153, CLASS C.		 A. THE SLOPE BETWEEN THE L HORIZONTAL, UNLESS NOTE EXCAVATION.
	 C. EXPANSION ANCHORS: HILTI KWIK BOLT 3 EXPANSION ANCHORS. INSTALL PER HILTI INSTALLATION RECOMMENDATIONS. UNLESS OTHERWISE NOTED EMBED BOLTS 10 BOLT DIAMETERS. D. POWDER-ACTUATED ANCHORS: MANUFACTURED BY HILTI OR APPROVED EQUAL. 		B. THE CONTRACTOR SHALL VI SYSTEM AS SHOWN PRIOR 1
	 E. MECHANICAL FASTENERS: ASTM C 1513, CORROSION-RESISTANT-COATED, SELF-DRILLING, SELF-TAPPING STEEL DRILL SCREWS. 1. HEAD TYPE: LOW-PROFILE HEAD BENEATH SHEATHING, MANUFACTURER'S STANDARD ELSEWHERE. 		AND BRACING WITH THE CIV INFORMATION NOT COVERE
	F. WELDING ELECTRODES: COMPLY WITH AWS STANDARDS.		C. THE CONTRACTOR SHALL B STRUCTURES, ETC., WHETH
5D.4	FRAMING SIZES: A. FRAMING MEMBERS SHALL BE OF THE TYPE AND GAUGE CALLED FOR ON THE DRAWINGS AND IN THE SPECIFICATIONS.		 D. UTILITIES LINES SHALL NOT UNLESS DETAILED OTHERW E. ALL SHORING, TEMPORARY
5D.5	FRAMING SIZES: A. THE CONTRACTOR SHALL DESIGN THE COLD FORMED METAL FRAMING SYSTEMS INDICATED ON THE DRAWINGS AND IN THE		BE THE TOTAL RESPONSIBIL
	SPECIFICATIONS AND SUBMIT SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS. THE FRAMING SYSTEMS SHALL CONFORM TO THE CONCEPTS INDICATED ON THE DRAWINGS AND WITH THE MINIMUM SIZES AND GAUGES INDICATED ON THE		SUBMITTALS FOR REVIEW.
	DRAWINGS OR IN THE SPECIFICATIONS. B. COLD-FORMED STEEL FRAMING DESIGN, GENERAL: DESIGN ACCORDING TO AISI'S "STANDARD FOR COLD-FORMED STEEL FRAMING		3 - FOUNDATIONS / EARTHWORK [/
	- GENERAL PROVISIONS." 1. HEADERS: DESIGN ACCORDING TO AISI'S "STANDARD FOR COLD-FORMED STEEL FRAMING - HEADER DESIGN."	31.1	REFERENCE GEOTECHNICAL REPO A. FOUNDATION DESIGN IS IN A [REFERENCE GEOTECHNICA
	 DESIGN EXTERIOR NON-LOAD-BEARING WALL FRAMING TO ACCOMMODATE HORIZONTAL DEFLECTION WITHOUT REGARD FOR CONTRIBUTION OF SHEATHING MATERIALS. ROOF TRUSSES: DESIGN ACCORDING TO AISI'S "STANDARD FOR COLD-FORMED STEEL FRAMING - TRUSS DESIGN." 		B. FOUNDATION DESIGN IS IN A
5D.6	GENERAL:		DOCUMENTS OR FROM THE GEOTECHNICAL REPORT FO
	A. DESIGN OF ALL COLD-FORMED METAL FRAMING SYSTEMS IN ACCORDANCE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS SHALL BE BY THE FABRICATOR'S ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION. CALCULATIONS AND		C. THE OWNER HAS NOT PROV FOUNDATIONS HAVE BEEN I
	SHOP DRAWINGS CONSISTING OF FRAMING PLANS, ELEVATIONS, AND DETAILS SHALL BE SUBMITTED BEARING THIS ENGINEER SEAL AND SIGNATURE.		PRESSURE PER THE BUILDIN STRUCTURAL DRAWINGS. D. ALL FOUNDATIONS SHALL B
	B. MEMBER DESIGNATIONS AND PROPERTIES ARE BASE ON DIETRICH INDUSTRIES, INC. STEEL FRAMING CATALOGUE. FRAMING BY OTHER MANUFACTURERS MAY BE SUPPLIED PROVIDED SECTION PROPERTIES EQUAL OR EXCEED THOSE SPECIFIED AND IF APPROVED BY THE ARCHITECT AND STRUCTURAL ENGINEER.		THESE REQUIREMENTS WIT ENGINEER IN ADVANCE OF A
	 C. CONTENTS OF THESE STRUCTURAL DOCUMENTS SHOW THE INTENDED APPLICATION OF COLD-FORMED FRAMING COMPONENTS. D. ALL DIMENSIONS SHOWN ON THE STRUCTURAL DOCUMENTS SHALL BE VERIFIED WITH THE ARCHITECTURAL AND MECHANICAL 		E. SEE THE SPECIFICATIONS A THE BEARING MATERIAL, AN
	DOCUMENTS PRIOR TO CONSTRUCTION OR FABRICATION. E. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS OR AS REQUIRED FOR AN		F. IN THE ABSENCE OF A GEOT
	ANGULAR FIT AGAINST ABUTTING MEMBERS. MEMBERS SHALL BE HELD POSITIVELY IN PLACE UNTIL PROPERLY FASTENED. F. ALL FIELD CUTTING OF STUDS MUST BE DONE BY SAWING OR SHEARING. TORCH CUTTING OF COLD-FORMED MEMBERS IS		G. REQUIREMENTS CONTAINED H. REFER TO GEOTECHNICAL F
	 UNACCEPTABLE. G. SPLICES IN STUDS, JOISTS, OR OTHER LOAD CARRYING MEMBERS ARE NOT PERMITTED. H. ALL STRUCTURAL COLD-FORMED METAL FRAMING (EXCLUDING STUDS) MEMBERS SHALL BE UN-PUNCHED UNLESS SPECIFICALLY 		I. IF UNSUITABLE BEARING MA
	 NOTED OTHERWISE. FOR AXIAL LOAD BEARING CONSTRUCTION, IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ADEQUATE WALL 	31.2	FOUNDATION DESIGN PARAMETER A. SPREAD FOOTINGS:
	BRACING/BRIDGING IS IN PLACE PRIOR TO LOADING THESE MEMBERS AND UNTIL SHEATHING IS PROPERLY ATTACHED TO BOTH STUD FLANGES. CONTRACTOR SHALL NOT OVERLOAD BEARING MEMBERS DURING CONSTRUCTION.		1. BUILDING SPREAD AN COMPACTED ENGINE B. SPREAD FOOTINGS ON GEO
	J. CONNECTIONS SHALL BE BY WELDING, SCREWING, OR OTHER APPROVED FASTENING DEVICES OR METHODS PROVIDING POSITIVE ATTACHMENT AND RESISTANCE TO LOOSENING. FASTENERS SHALL BE OF COMPATIBLE MATERIAL. WHENEVER POSSIBLE,		1. BUILDING SPREAD AN PRESSURE OF 2,000 F
	CONNECTIONS SHALL FOLLOW THE RECOMMENDATIONS MADE BY THE METAL LATH AND STEEL FRAMING ASSOCIATION. THE CONTRACTOR SHALL CONFIRM THAT THE FASTENERS THEY INTEND TO USE MEETS OR EXCEEDS THE DESIGN VALUES SHOWN IN THE SUBMITTED CALCULATIONS.	31.3	EXCAVATION:
	 K. TOUCH-UP ALL WELDS (IF USED) WITH ZINC RICH PAINT. L. FOR FASTENERS PROVIDE THE MINIMUM CLEARANCES, FASTENER SPACING, AND EDGE DISTANCE AS NOTED BELOW, UNLESS 		A. THE SLOPE BETWEEN THE L HORIZONTAL, UNLESS NOTE
	OTHERWISE NOTED OR DETAILED:		EXCAVATION. B. THE CONTRACTOR SHALL VI SYSTEM AS SHOWN PRIOR 1
	FASTENER TYPESPACINGEDGE DISTANCE1.SCREWS:½ IN.½ IN.2.POWDER DRIVEN FASTENERS (STEEL):1½ IN.½ IN.		AND BRACING WITH THE CIV
	2.POWDER DRIVEN FASTENERS (STEEL):1½ IN.½ IN.3.POWDER DRIVEN FASTENERS (CONCRETE):4 IN.3 IN.		C. THE CONTRACTOR SHALL B STRUCTURES, ETC., WHETH
	M. ALTERNATE CONNECTION DESIGNS SHALL ONLY BE ALLOWED WITH PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. IF SUCH APPROVAL IS GRANTED, ALL CONNECTIONS AND/OR DETAILS NOT IN ACCORDANCE WITH THE CONTRACT DOCUMENTS SHALL BE		D. UTILITIES LINES SHALL NOT UNLESS DETAILED OTHERW
	PREPARED BY THE CONTRACTOR'S ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION. CALCULATIONS AND SHOP DRAWINGS SHALL BE SUBMITTED BEARING THE ENGINEER'S SEAL AND SIGNATURE.		E. ALL SHORING, TEMPORARY BE THE TOTAL RESPONSIBIL JURISDICTION, SHALL DESIG
	N. DESIGN OF TRUSSES, TRUSS TEMPORARY AND PERMANENT BRACING, AND DETAILING OF TRUSS CONNECTIONS IN ACCORDANCE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS SHALL BE BY THE FABRICATOR'S ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION. CALCULATIONS AND SHOP DRAWINGS CONSISTING OF TRUSS LAYOUT PLANS AND DETAILS SHALL BE		SUBMITTALS FOR REVIEW.
	 SUBMITTED BEARING THIS ENGINEER'S SEAL AND SIGNATURE. THE CONTRACTOR SHALL SUBMIT FOR REVIEW DRAWINGS AND CALCULATIONS, SIGNED AND SEALED BY A STRUCTURAL ENGINEER 	31.4	BACKFILL UNDER SLAB ON GRADE A. BACKFILL WHERE REQUIRED
	REGISTERED IN THE PROJECT'S JURISDICTION, FOR THE EXTERIOR COLD-FORMED METAL CURTAIN WALL SYSTEM AND RELATED CONNECTIONS. THE DESIGN SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY THE APPLICABLE		B. FOLLOWING REQUIRED STR
	BUILDING CODES. THIS REVIEW SHALL BE FOR GENERAL CONFORMANCE WITH THE PROJECTS PARAMETERS AS INDICATED ON THE DRAWINGS AND IN THE GENERAL NOTES. THE DESIGN OF THIS SYSTEM IS THE RESPONSIBILITY OF THE ENGINEER WHO HAS		GEOTECHNICAL ENGINEER. SOILS REQUIRING IMPROVE COMPACTED MATERIALS.
	 SIGNED AND SEALED THE SHOP DRAWINGS AND CALCULATIONS. BACK UP SYSTEMS AND CURTAIN WALLS SHALL BE DESIGNED FOR A MAXIMUM DEFLECTION OF L/600 OF THE SPAN IN INCHES, OR ³/₄ IN., WHICHEVER IS LESS, FOR THE APPLICABLE DESIGN WIND LOAD. THE SUBMITTED CALCULATIONS SHALL 	31.5	BACKFILL AGAINST WALLS:
	CLEARLY SHOW THE LOAD REACTIONS AS APPLIED TO THE BUILDING STRUCTURE.		A. DO NOT BACKFILL AGAINST MATERIAL PLACED IN 6 IN. L
	-044DL-CONA I		DEFINED BY ASTM D-1557, M B. NO BACKFILL MATERIAL SHA STRENGTH AND THE UPPER
	POLSHEETS/23		CONTRACTOR'S ENGINEER, AND SEALED SUBMITTALS F
	CO/DO		C. WHERE THE FINAL GRADE E LEVEL ELEVATIONS WITHIN
	A Honda A Mine A Mine Mine Mine Mine	31.6	FOUNDATION PLACEMENT & PROT A. DO NOT PLACE FOUNDATION
	23-044DL-C.M.		FROST PENETRATION UNTIL B. NEW FOOTING BEARING ELE
	, PO/Projects		C. CONCRETE FOR FOUNDATIC
	Ag Consultants		D. BEARING ELEVATIONS INDIC GEOTECHNICAL REPORT. P
			FIELD VERIFY ALLOWABLE B
	e Date - Tare - Tar	31.7	STRUCTURAL FILL: A. REFER TO SPECIFICATIONS
	h.gonzalez\On		CONTAINED IN THE GEOTEC STRUCTURAL FILL SHALL BE

31.8 BELOW GRADE WALL DRAINAGE:

PART 3 - FOUNDATIONS / EARTHWORK / GEOTECHNICAL REPORT

HNICAL REPORT:

ESIGN IS IN ACCORDANCE WITH THE RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT PREPARED BY , DATED MARCH 24, 2023 AUTHOR'S REPORT NUMBER 07-23-0022.

HAVE BEEN DESIGNED WITH A BEARING CAPACITY OF 2,000 PSF PER THE MAXIMUM ALLOWED BEARING PRESSURE ECHNICAL REPORT.

ONS SHALL BEAR A MINIMUM OF 36" BELOW ADJACENT EXTERIOR GRADE. THE CONTRACTOR SHALL COORDINATE EMENTS WITH ALL UNDERGROUND UTILITIES, TUNNELS, ETC. AND NOTIFY THE ARCHITECT AND STRUCTURAL DVANCE OF ANY CONSTRUCTION TO ALLOW FOR ADJUSTMENTS.

FICATIONS AND GEOTECHNICAL REPORT REQUIREMENTS FOR EXCAVATION, SUITABILITY AND REPLACEMENT OF MATERIAL, AND PREPARATION OF THE SUBGRADE FOR THE FOUNDATIONS AND THE SLAB ON GRADE, INCLUDING PROCEDURES.

E OF A GEOTECHNICAL REPORT AND DIRECTION, IF UNSUITABLE BEARING MATERIAL IS FOUND, THE POOR ULD BE REMOVED AND REPLACED WITH LEAN CONCRETE.

CONTAINED IN THE GEOTECHNICAL REPORT ARE PART OF THIS WORK/ CONTRACT DOCUMENTS. TECHNICAL REPORT FOR UNDER-SLAB DRAINAGE SYSTEM.

BEARING MATERIAL IS DISCOVERED THE GEOTECHNICAL ENGINEER SHALL BE ALERTED

PARAMETERS

NGS: SPREAD AND STRIP FOOTINGS SHALL BEAR ON UNDISTURBED NATURAL SOILS OR PROPERLY PLACED AND TED ENGINEERED FILL WITH AN ALLOWABLE BEARING PRESSURE OF2,000 PSF.

IWEEN THE LOWER EDGES OF ADJACENT FOUNDATIONS SHALL NOT EXCEED 30 DEGREES REFERENCED FROM THI JNLESS NOTED OR DETAILED OTHERWISE ON THE PLAN. MAINTAIN A 1V:2H SLOPE FROM BOTTOM EDGE OF ANY

FOR SHALL VERIFY ALL EXISTING FIELD CONDITIONS THAT MAY AFFECT THE INSTALLATION OF THE FOUNDATION OWN PRIOR TO STARTING WORK. CONTRACTOR SHALL COORDINATE THE EXTENT OF THE EXCAVATION, SHORING WITH THE CIVIL ENGINEERS'S DRAWING AND REFER TO THOSE DRAWINGS AND SPECIFICATIONS FOR RELATED NOT COVERED IN THE STRUCTURAL DRAWINGS.

FOR SHALL BE RESPONSIBLE FOR LOCATING AND PROTECTING ALL EXISTING UTILITIES, ABOVE AND BELOW GRADE ETC., WHETHER INDICATED OR NOT, THAT MAY BE AFFECTED BY THE CONSTRUCTION PROCESS. SHALL NOT BE PLACED THROUGH OR BELOW FOUNDATIONS WITHOUT THE STRUCTURAL ENGINEER'S APPROVAL

ED OTHERWISE IN THE PLANS. TEMPORARY BRACED EXCAVATION (INCLUDING STAGED UNDERPINNING PITS), SHEETING AND DEWATERING SHALI RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR'S ENGINEER, REGISTERED IN THE PROJECT'S SHALL DESIGN THE SHEETING AND SHORING, AND BRACED EXCAVATION AND PROVIDE SIGNED AND SEALED OR REVIEW.

RTHWORK [/ GEOTECHNICAL REPORT]

HNICAL REPORT:

ESIGN IS IN ACCORDANCE WITH THE RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT PREPARED BY TECHNIQUES. EOTECHNICAL ENGINEER], DATED [REFERENCE DATE OF REPORT], AUTHOR'S REPORT NUMBER [REFERENCE **REPORT REFERENCE NUMBER].**

ESIGN IS IN ACCORDANCE WITH THE GEOTECHNICAL INFORMATION SHOWN ON THE EXISTING CONSTRUCTION R FROM THE PRIOR GEOTECHNICAL REPORT NOTED ABOVE. THE OWNER HAS NOT PROVIDED A NEW REPORT FOR THIS PROJECT AND THUS HAS ACCEPTED THE RISK.

AS NOT PROVIDED A GEOTECHNICAL REPORT FOR THIS PROJECT AND THUS HAS ACCEPTED THE RISK. HAVE BEEN DESIGNED WITH AN ASSUMED BEARING CAPACITY OF PSF PER THE MAXIMUM ALLOWED BEARING THE BUILDING CODE WITHOUT A GEOTECHNICAL REPORT/ PER THE INFORMATION FOUND ON AS-BUILT

RAWINGS. DNS SHALL BEAR A MINIMUM OF BELOW ADJACENT EXTERIOR GRADE. THE CONTRACTOR SHALL COORDINAT EMENTS WITH ALL UNDERGROUND UTILITIES, TUNNELS, ETC. AND NOTIFY THE ARCHITECT AND STRUCTURAL

DVANCE OF ANY CONSTRUCTION TO ALLOW FOR ADJUSTMENTS. FICATIONS AND GEOTECHNICAL REPORT REQUIREMENTS FOR EXCAVATION, SUITABILITY AND REPLACEMENT OF MATERIAL, AND PREPARATION OF THE SUBGRADE FOR THE FOUNDATIONS AND THE SLAB ON GRADE, INCLUDING

PROCEDURES. E OF A GEOTECHNICAL REPORT AND DIRECTION, IF UNSUITABLE BEARING MATERIAL IS FOUND, THE POOR ULD BE REMOVED AND REPLACED WITH LEAN CONCRETE.

CONTAINED IN THE GEOTECHNICAL REPORT ARE PART OF THIS WORK/ CONTRACT DOCUMENTS.

TECHNICAL REPORT FOR UNDER-SLAB DRAINAGE SYSTEM. BEARING MATERIAL IS DISCOVERED THE GEOTECHNICAL ENGINEER SHALL BE ALERTED

PARAMETERS

SPREAD AND STRIP FOOTINGS SHALL BEAR ON UNDISTURBED NATURAL SOILS OR PROPERLY PLACED AND TED ENGINEERED FILL WITH AN ALLOWABLE BEARING PRESSURE OF 2.000 PSF.

NGS ON GEO-PIERS: SPREAD AND STRIP FOOTINGS SHALL BEAR ON PROPERLY PLACED GEO-PIERS WITH AN ALLOWABLE BEARING RE OF 2,000 PSF.

IWEEN THE LOWER EDGES OF ADJACENT FOUNDATIONS SHALL NOT EXCEED 30 DEGREES REFERENCED FROM THE JNLESS NOTED OR DETAILED OTHERWISE ON THE PLAN. MAINTAIN A 1V:2H SLOPE FROM BOTTOM EDGE OF ANY

TOR SHALL VERIFY ALL EXISTING FIELD CONDITIONS THAT MAY AFFECT THE INSTALLATION OF THE FOUNDATION OWN PRIOR TO STARTING WORK. CONTRACTOR SHALL COORDINATE THE EXTENT OF THE EXCAVATION, SHORING WITH THE CIVIL ENGINEERS'S DRAWING AND REFER TO THOSE DRAWINGS AND SPECIFICATIONS FOR RELATED NOT COVERED IN THE STRUCTURAL DRAWINGS.

TOR SHALL BE RESPONSIBLE FOR LOCATING AND PROTECTING ALL EXISTING UTILITIES, ABOVE AND BELOW GRADE ETC., WHETHER INDICATED OR NOT, THAT MAY BE AFFECTED BY THE CONSTRUCTION PROCESS. S SHALL NOT BE PLACED THROUGH OR BELOW FOUNDATIONS WITHOUT THE STRUCTURAL ENGINEER'S APPROVAL ED OTHERWISE IN THE PLANS.

TEMPORARY BRACED EXCAVATION (INCLUDING STAGED UNDERPINNING PITS), SHEETING AND DEWATERING SHALL RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR'S ENGINEER, REGISTERED IN THE PROJECT'S SHALL DESIGN THE SHEETING AND SHORING, AND BRACED EXCAVATION AND PROVIDE SIGNED AND SEALED OR REVIEW.

RE REQUIRED BELOW SLABS WITH APPROVED GRANULAR SOIL PLACED IN 6 IN. LAYERS AND COMPACTED TO 95% TIMUM MOISTURE CONTENT AS DEFINED BY ASTM D-1557, METHOD D.

EQUIRED STRIPPING OPERATIONS, ANY PROOFROLLING SHALL BE AS DIRECTED BY AN EXPERIENCED, QUALIFIED L ENGINEER. THE PURPOSE OF PROOFROLLING WILL BE TO LOCATE ANY ISOLATED AREAS OF SOFT OR LOOSE NG IMPROVEMENT OR REPLACEMENT. SOFT AREAS SHALL BE UNDERCUT AND REPLACED WITH PROPERLY IATERIALS.

ALLS:

ILL AGAINST RETAINING WALLS UNTIL WALL CONCRETE IS AT FULL DESIGN STRENGTH. BACKFILL WITH APPROVED CED IN 6 IN. LAYERS AND COMPACTED TO 95% DENSITY AT OPTIMUM MOISTURE CONTENT AND FREE OF DEBRIS AS TM D-1557, METHOD D.

ATERIAL SHALL BE PLACED AGAINST FOUNDATION WALLS UNTIL THE WALL HAS ATTAINED 75% OF ITS DESIGN) THE UPPER BRACING FLOORS ARE IN PLACE FOR AT LEAST 7 DAYS, OR ADEQUATE BRACING IS INSTALLED. THE SENGINEER, REGISTERED IN THE PROJECT'S JURISDICTION, SHALL DESIGN THE BRACING AND PROVIDE SIGNED UBMITTALS FOR REVIEW.

VAL GRADE ELEVATIONS ARE APPROXIMATELY EQUAL ON BOTH SIDES OF A WALL, BACKFILL IN LIFTS TO MAINTAIN ONS WITHIN 12 IN. ON BOTH SIDES AT ANY TIME.

IENT & PROTECTION:

FOUNDATION CONCRETE IN WATER OR ON FROZEN GROUND. PROTECT IN-PLACE FOUNDATIONS AND SLABS FROM RATION UNTIL THE PROJECT IS COMPLETE. DO NOT USE SALT OR CHLORIDE COMPOUNDS TO DE-ICE THE SITE. BEARING ELEVATION IS TO MATCH ADJACENT EXISTING FOOTING BEARING ELEVATION WHERE APPLICABLE UNLESS AILED OTHERWISE ON PLAN.

R FOUNDATIONS SHALL BE POURED ON THE SAME DAY SUBGRADE APPROVAL IS GIVEN BY THE GEOTECHNICAL

ATIONS INDICATED ON THE DRAWINGS ARE ESTIMATED FROM SOIL BEARING DATA INDICATED IN THE REPORT. PRIOR TO PLACING FOUNDATIONS. AN EXPERIENCED. QUALIFIED GEOTECHNICAL ENGINEER SHALL LLOWABLE BEARING PRESSURES AND DETERMINE FINAL BEARING ELEVATIONS.

CIFICATIONS AND GEOTECHNICAL REPORT REQUIREMENTS FOR COMPACTED STRUCTURAL FILL. REQUIREMENTS THE GEOTECHNICAL REPORT ARE PART OF THIS WORK. INSPECTION OF THE PLACEMENT OF COMPACTED FILL SHALL BE BY AN EXPERIENCED, QUALIFIED GEOTECHNICAL ENGINEER.

A. THE BELOW-GRADE AREAS FOR THE STRUCTURE SHOULD BE PROVIDED WITH A PERIMETER DRAINAGE SYSTEM BELOW-GRADE WALL WATERPROOFING SEE ARCH DWGS.

1704.3 STATEMENT OF SPECIAL INSPECTIONS.

THE CONTRACTOR OR BUILDING OWNER SHALL RETAIN AN APPROVED THIRD PARTY AGENCY TO PERFORM SPECIAL INSPECTIONS. SPECIAL INSPECTIONS AND REPORTING SHALL CONFORM 1 CHAPTER 17 OF THE 2018 INTERNATIONAL BUILDING CODE.

1704.2.5 SPECIAL INSPECTION OF FABRICATED ITEMS. WHERE FABRICATION OF STRUCTURAL, LOAD-BEARING OR LATERAL LOAD RESISTING MEMBERS OR ASSEMBLIES IS BEING CONDUCTED O THE PREMISES OF A FABRICATOR'S SHOP, SPECIAL INSPECTIONS OF THE FABRICATED ITEMS SHALL BE PERFORMED DURING FABRICATION.

ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360.

TABLE 1705.3

REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION

. INSPECTION OF REINFORCING STEEL.

2. REINFORCING BAR WELDING:

a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A 706; . INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND

. INSPECT ALL OTHER WELDS.

. INSPECT ANCHORS CAST IN CONCRETE.

4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS . ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLI ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.

. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.

5. VERIFY USE OF REQUIRED DESIGN MIX.

6. PIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGT TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE

7. INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION FECHNIQUES.

8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND

. ERECTION OF PRECAST CONCRETE MEMBERS.

10. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.

1705.6 SOILS. SPECIAL INSPECTIONS AND TESTS OF EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT AND LOAD-BEARING REQUIREMENTS SHALL BE PERFORMED IN ACCORDANCE WITH THIS SECTION AND TABLE 1705.6. THE APPROVED GEOTECHNICAL REPORT AND THE CONSTRUCTION DOCUMENTS PREPARED BY THE REGISTERED DESIGN PROFESSIONALS SHALL BE USED TO DETERMINE COMPLIANCE. DURING FILL PLACEMENT, THE SPECIAL INSPECTOR SHALL VERIFY THAT PROPER MATERIALS AND PROCEDURES ARE USED IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT.

TABLE 1705.6 REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS

VERIFICATION AND INSPECTION

. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY, BY INSTALLATION OF RAMMED AGGREGATE PIERS.

2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED

PROPER MATERIAL. 3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIAL

4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES

DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.

. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.

1705.4 MASONRY CONSTRUCTION. SPECIAL INSPECTIONS AND TESTS OF LEVEL C MASONRY CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE PROGRAM REQUIREMENTS OF TMS 402/ACI 530/ASCE 5 AND TMS 602/ACI 530.1/ASCE 6.

1705.11.1 STRUCTURAL WOOD. CONTINUOUS SPECIAL INSPECTION IS REQUIRED DURING FIELD GLUING OPERATIONS OF ELEMENTS OF THE MAIN WINDFORCE-RESISTING SYSTEM. PERIODIC SPE INSPECTION IS REQUIRED FOR NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF ELEMENTS OF THE MAIN WINDFORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES AND HOLD-DOWNS.

	-,							
ABBREVIATION	WORD OR PHRASE	ABBREVIATION	WORD OR PHRASE	ABBREVIATION	WORD OR PHRASE			
ASD ACI	ALLOWABLE STRESS DESIGN AMERICAN CONCRETE INSTITUTE	FT	FEET	REF REINF	REFERENCE REINFORCE OR REINFORCEMENT	l certify th am a d	at these documents were prepa uly licensed engineer under the	red or approved by me, and that I laws of the Commonwealth of xpiration date: 12-31-2023.
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	GALV	GALVANIZED			· · · · ·		xpiration date: 12-51 2025.
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS		GAGE	SECT	SECTION			
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS	GR	GRADE	SIM	SIMILAR			
AWS	AMERICAN WELDING SOCIETY	•••		SOG	SLAB-ON-GRADE			
ARCH	ARCHITECT	HSS	HOLLOW STRUCTURAL SECTION	SQ	SQUARE			
@	AT RATE OF	1100		STD	STANDARD		+	
e		IN	INCH	STL	STEEL		+	
BP	BASE PLATE	INFO	INFORMATION	SDI	STEEL DECK INSTITUTE			
B OR BOT	BOTTOM	IBC	INTERNATIONAL BUILDING CODE	STIFF	STIFFENER			
BM	BEAM			51111	JTHTERER			
DIM	DEAM	JT	JOINT	т	ТОР			
CANT	CANTILEVER	51	50111	, TYP	TYPICAL			
CIP	CAST-IN-PLACE	К	KIP (1000 POUNDS)	111	TTTCAL		1	
CL	CENTERLINE	KSF	KIPS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE		1	
CLR	CLEAR	KSI	-	UNU			+	
	CONCRETE	ICN	KIPS PER SQUARE INCH	V OR VERT	VERTICAL			
CONC		1.147		V OR VERI	VERTICAL			
CMU	CONCRETE MASONRY UNIT	LW					Bid Set	2023.07.27
CRSI	CONCRETE REINFORCING STEEL INSTITUTE	LRFD	LOAD & RESISTANCE FACTOR	WWR		No.	Issue / Revision	Date
CONT	CONTINUOUS	DESIGN		W/	WITH	Drawr	ו By:	HAG
DI 4		LLH	LONG LEG HORIZONTAL			Check	ed By:	MWD
DIA	DIAMETER	LLV	LONG LEG VERTICAL			Plot D	•	July 28, 2023
DIM	DIMENSION							,,
DWLS	DOWELS	MFR	MANUFACTURER				Sheet Nur	mher
DWG	DRAWING	MATL	MATERIAL				Sheet Hu	linder
		MAX	MAXIMUM					$ \mathbf{n} \mathbf{n} $
EA	EACH	MIN	MINIMUM				S-0	
EE	EACH END						JU	UJ
EF	EACH FACE	NTS	NOT TO SCALE					
ES	EACH SIDE	NO OR #	NUMBER					
EW	EACH WAY						Sheet Ti	itle
EL	ELEVATION	0C	ON CENTER				GENE	Γ ΑΙ
EQ	EQUAL						ULINL	
EX OR (E)	EXISTING	LB OR #	POUND				NOT	-EC
. /		PSF	POUNDS PER SQUARE FOOT				INOT	LJ
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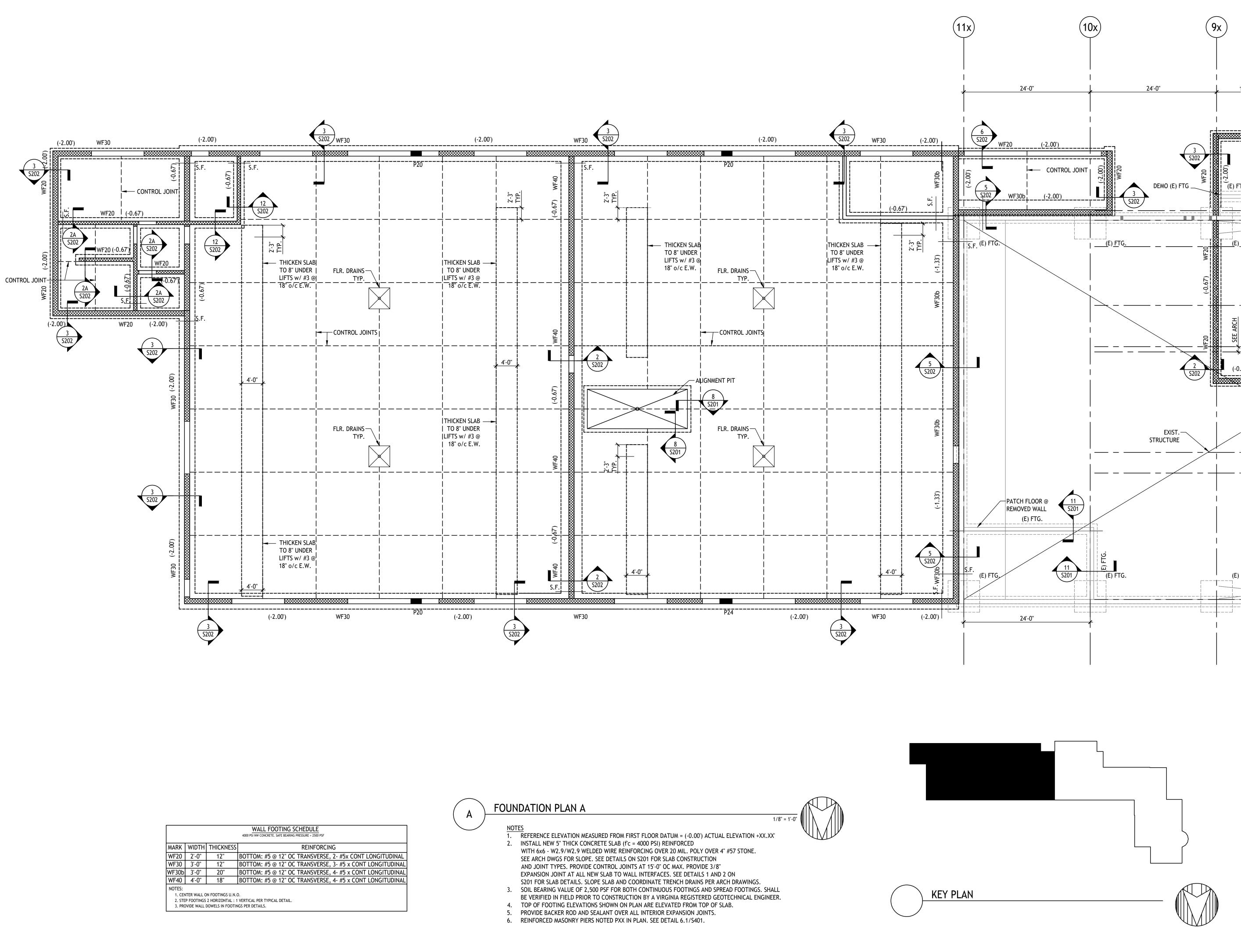
1705.2.1 STRUCTURAL STEEL. SPECIAL INSPECTIONS AND NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN BUILDINGS, STRUCTURES AND PORTIONS THEREOF SHALL BE IN

1705.3 CONCRETE CONSTRUCTION. SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH THIS SECTION AND TABLE 1705.3.

	FREQUENCY O	F INSPECTION	REFERENCE	CRITERIA
	CONTINUOUS	PERIODIC	REFERENCE STANDARD	IBC REFERENCE
	-	Х	ACI 318 CH. 20, 25.2, 25.3, 26.5.1-26.5.3	1908.4
;	- - X	X X -	AWS D1.4 ACI 318:26.5.4	-
	-	Х	ACI 318: 17.8.2	-
RS. _INED	X -	- X	ACI 318: 17.8.2.4 ACI 318: 17.8.2	-
	-	Х	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
TH	Х	-	ASTM C 172 ASTM C 31 ACI 318: 26.4.5, 26.12	1908.10
	Х	-	ACI 318: 26.4.5	1908.6, 1908.7, 1908.8
	-	Х	ACI 318: 26.4.7-26.4.9	1908.9
	-	Х	ACI 318: CH. 26.8	-
IE		X	ACI 318: 26.10.1(b)	-
			•	

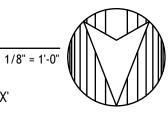
	FREQUENCY O	F INSPECTION
	CONTINUOUS	PERIODIC
	-	Х
	-	Х
.S.	-	Х
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	-	Х

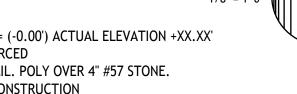
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908.2,		CMA Honda Winchester	Addition & Alteration		3985 Valley Pike	Winchester, VA	:
			3951 V	MA alley Pike ester, VA			
ECIAL							
٩T	am a du	t these docum	gineer unde	r the laws o	f the Co	mmonwea	lth of
IT	No. Drawn Checke Plot Da	ed By:		Numbe		July 28	07.27 Date HAG MWD , 2023

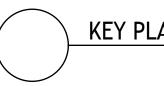


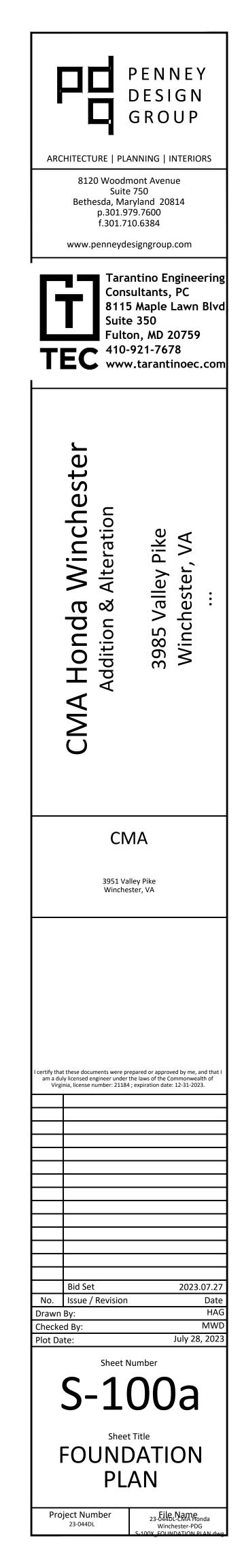
	WALL FOOTING SCHEDULE 4000 PSI NW CONCRETE. SAFE BEARING PRESSURE = 2500 PSF				
MARK	WIDTH	THICKNESS	REINFORCING		
WF20	2'-0"	12"	BOTTOM: #5 @ 12" OC TRANSVERSE, 2- #5x CONT LONGITUDINAL		
WF30	3'-0"	12"	BOTTOM: #5 @ 12" OC TRANSVERSE, 3- #5 x CONT LONGITUDINAL		
WF30b	3'-0"	20"	BOTTOM: #5 @ 12" OC TRANSVERSE, 4- #5 x CONT LONGITUDINAL		
WF40	4'-0"	18"	BOTTOM: #5 @ 12" OC TRANSVERSE, 4- #5 x CONT LONGITUDINAL		
NOTES:	NOTES:				
1. CEN	1. CENTER WALL ON FOOTINGS U.N.O.				
2. STE	2. STEP FOOTINGS 2 HORIZONTAL : 1 VERTICAL PER TYPICAL DETAIL.				
3. PRC	3. PROVIDE WALL DOWELS IN FOOTINGS PER DETAILS.				

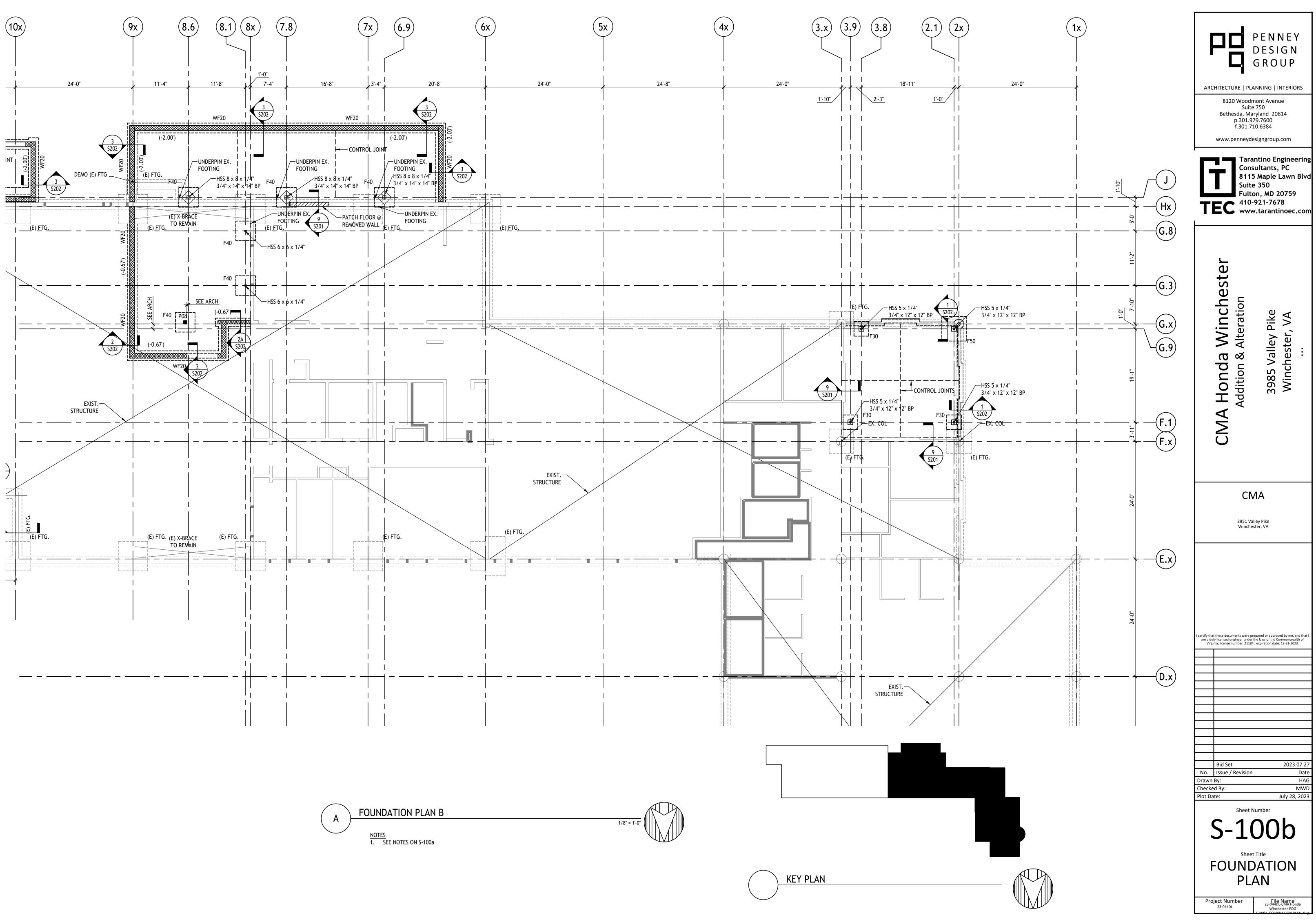




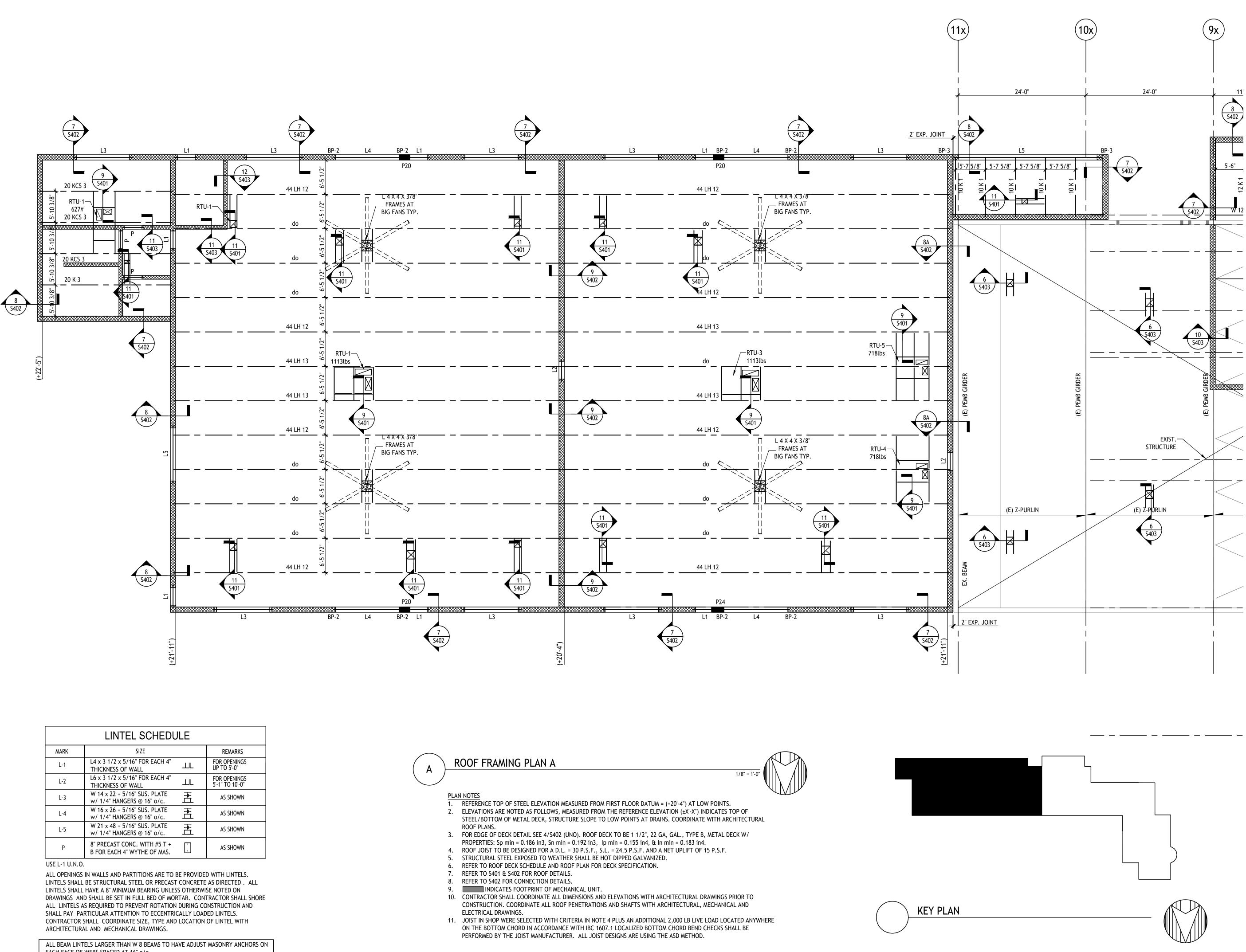






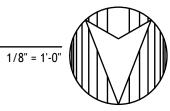




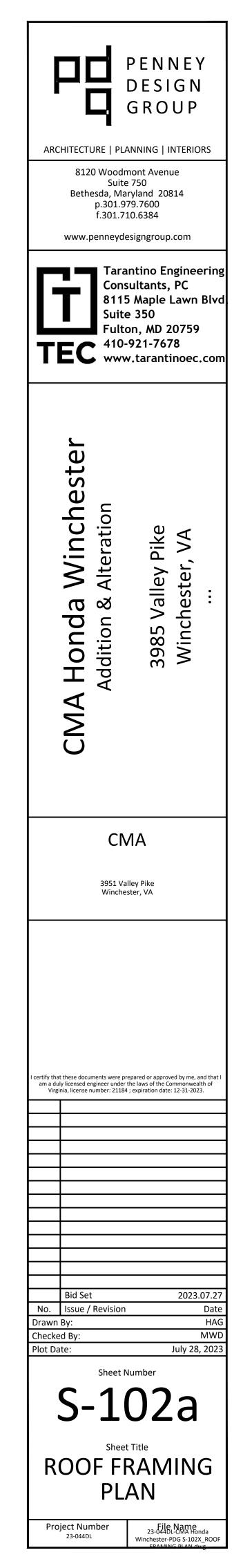


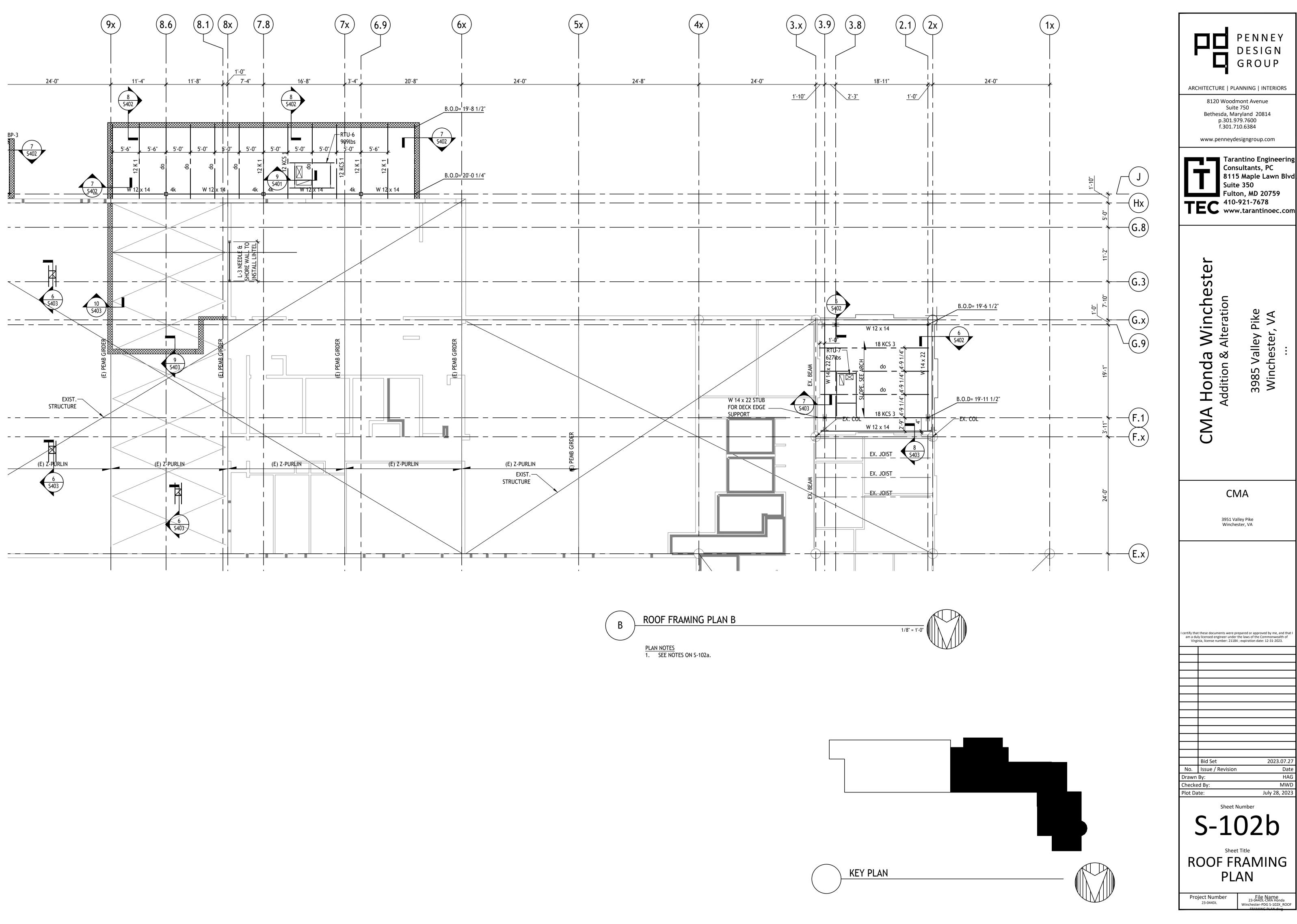
	LINTEL SCHEDULE				
MARK	SIZE		REMARKS		
L-1	L4 x 3 1/2 x 5/16" FOR EACH 4" THICKNESS OF WALL		For openings UP to 5'-0"		
L-2	L6 x 3 1/2 x 5/16" FOR EACH 4" THICKNESS OF WALL		FOR OPENINGS 5'-1" TO 10'-0"		
L-3	W 14 x 22 + 5/16" SUS. PLATE w/ 1/4" HANGERS @ 16" o/c.	Ŧ	AS SHOWN		
L-4	W 16 x 26 + 5/16" SUS. PLATE w/ 1/4" HANGERS @ 16" o/c.	Ŧ	AS SHOWN		
L-5	W 21 x 48 + 5/16" SUS. PLATE w/ 1/4" HANGERS @ 16" o/c.	Ŧ	AS SHOWN		
Р	8" PRECAST CONC. WITH #5 T + B FOR EACH 4" WYTHE OF MAS.	Ċ	AS SHOWN		

EACH FACE OF WEBS SPACED AT 16" o/c.

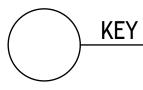


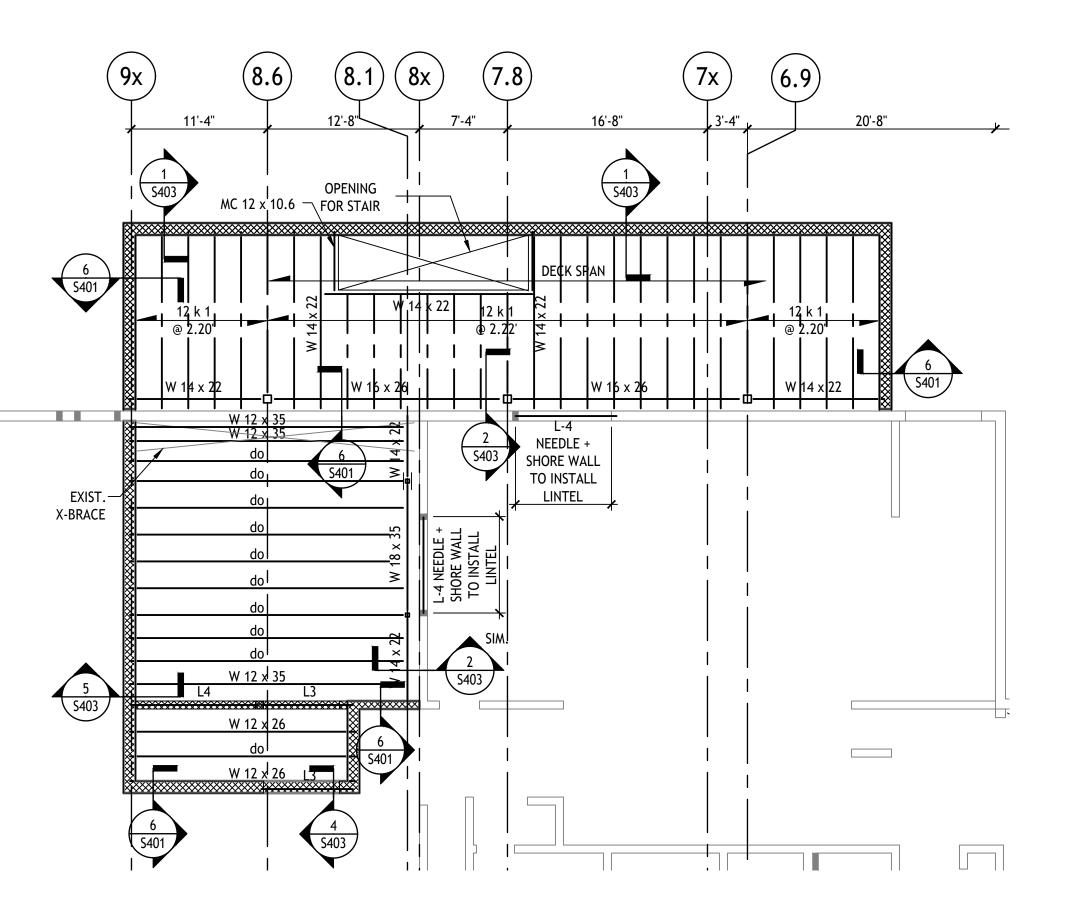










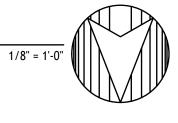


MEZZANINE FRAMING PLAN

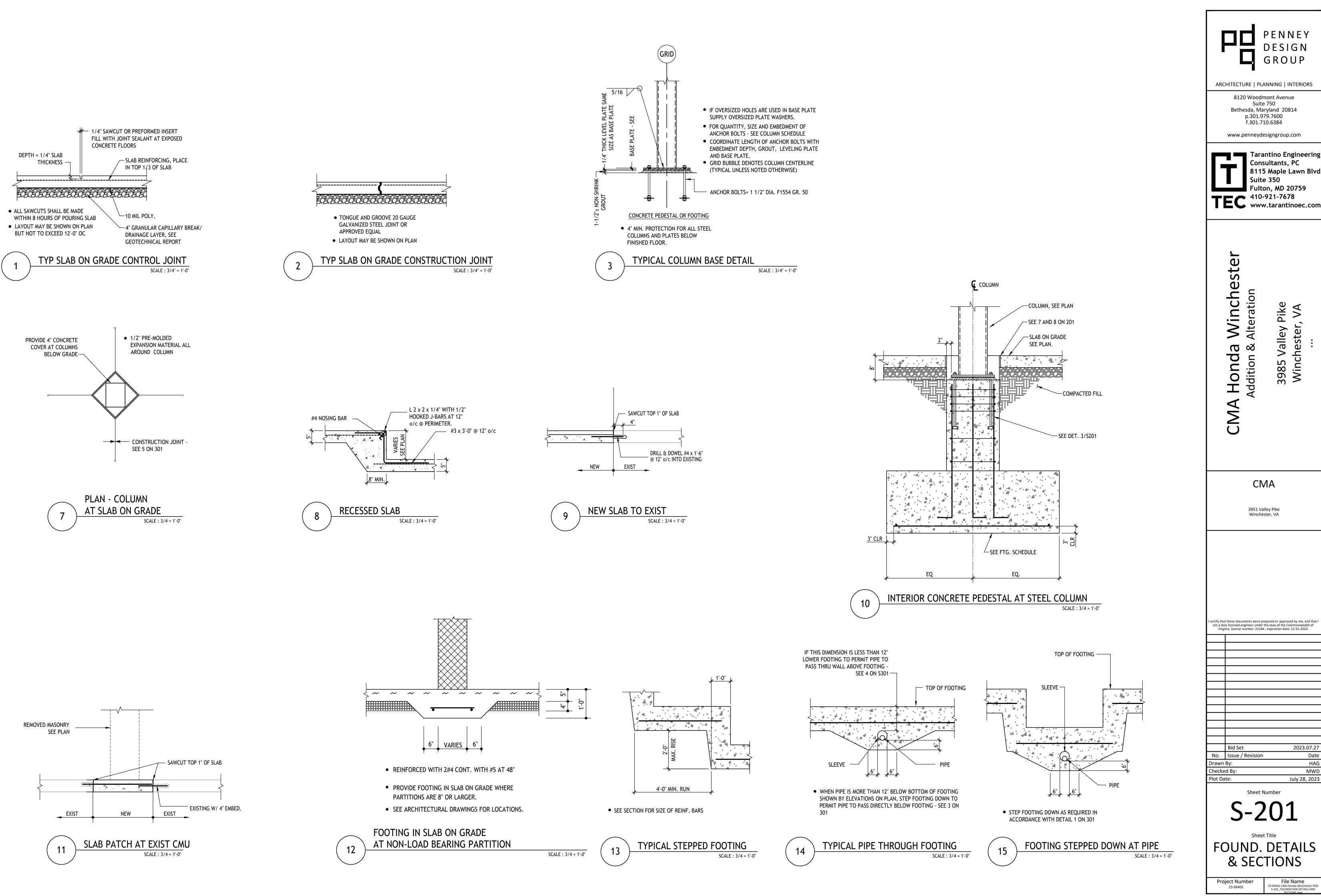
PLAN NOTES:

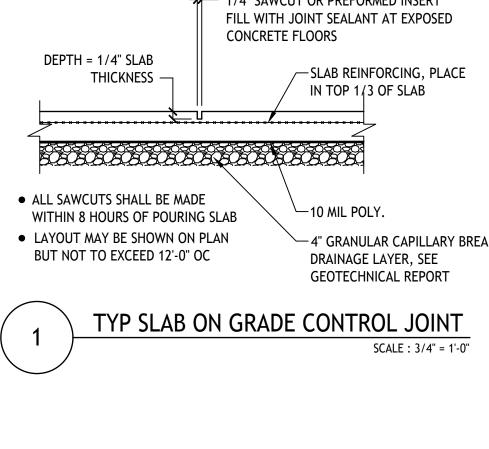
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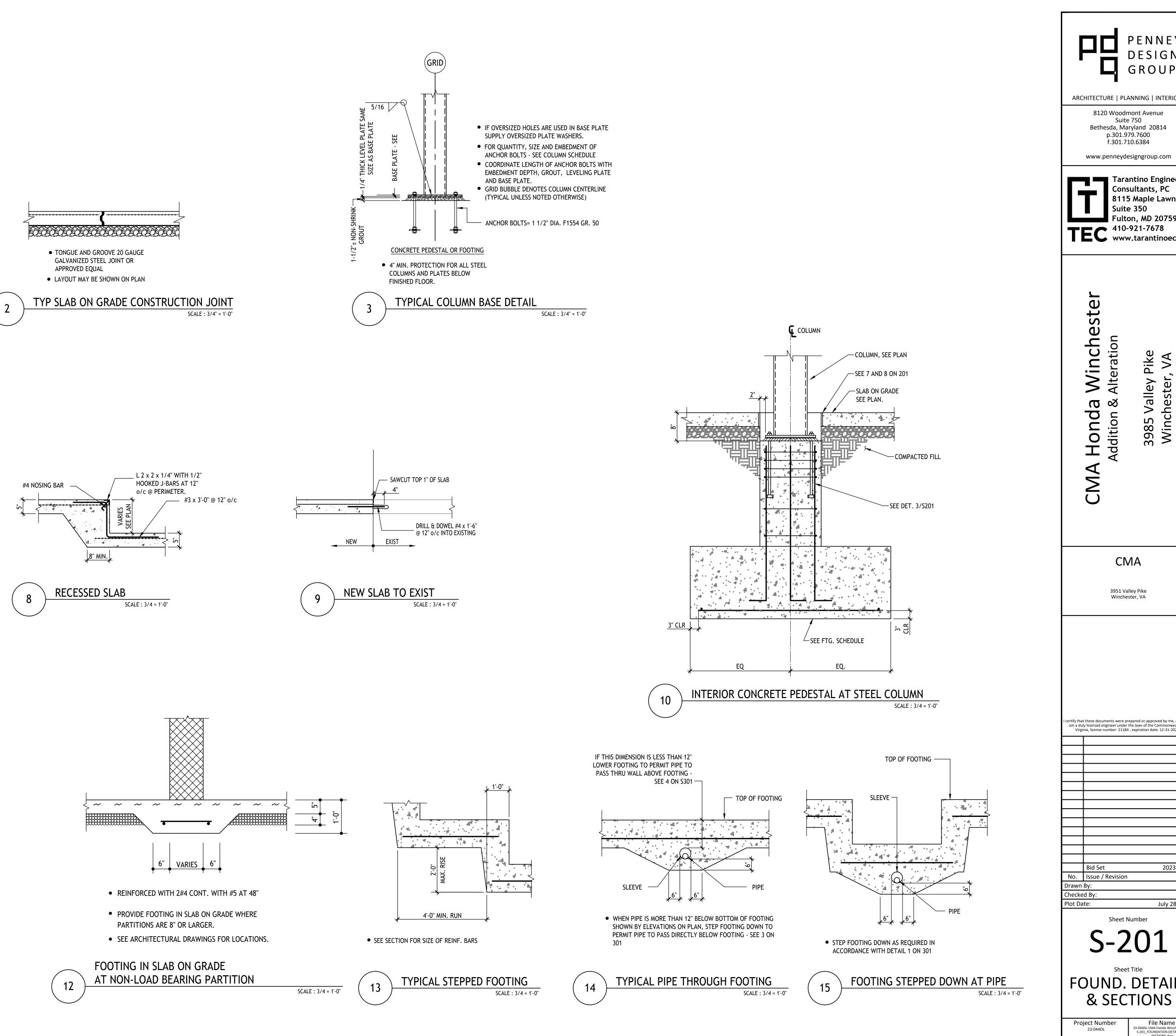
- 1. REFERENCE ELEVATION MEASURED FROM FIRST FIRST FLOOR DATUM = (+10'-0") = U.N.O.
- 2. MEZZ. FLOOR DESIGNED FOR 150 PSF LIVE LOAD.
- 3. ELEVATIONS ARE NOTED AS FOLLOWS, MEASURED FROM THE REFERENCE ELEVATION:
- T/SLAB (±X'-X)INDICATES TOP OF CONCRETE SLAB. TOP OF CONCRETE SLAB SHALL BE AT T/SLAB (+0'-0"), U.N.O.(±X'-X")INDICATES TOP OF STEEL/BOTTOM OF METAL DECK. TOP OF STEEL BE AT (-0'-3"), UNLESS OTHERWISE NOTED. 4. STRUCTURAL FLOOR SLAB SHALL BE 3" NORMAL WT. CONCRETE REINFORCED W/ 6" X 6" - W1.4/W1.4 WWR AND POURED OVER 9/16", 26GA, GALV.
- FORM METAL DECK. (SLAB + DECK TOTAL = 3" THICK).



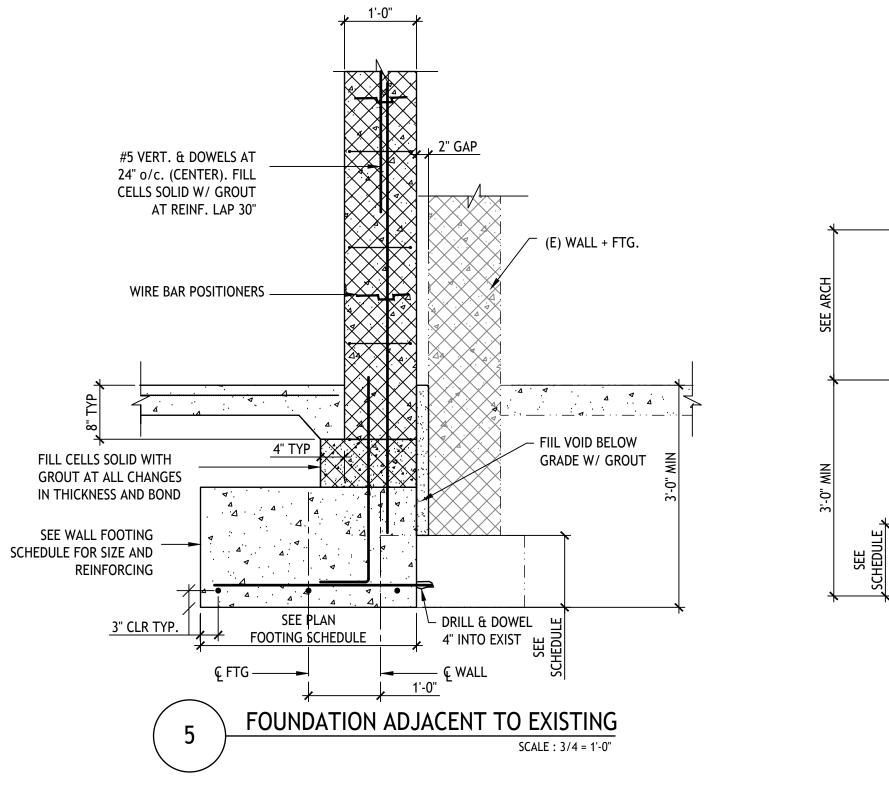
PERNNEY DESIGN GROUPARCHITECTURE PLANNING INTERIORSARCHITECTURE PLANNING INTERIORS8120 Woodmont Avenue Suite 750Bethesda, Maryland 20814 p.301.979.7600 f.301.710.6384www.penneydesigngroup.comTarantino Engineering Consultants, PC 8115 Maple Lawn Blvd Suite 350 Fulton, MD 20759 410-921-7678 www.tarantinoec.com			
CMA Honda Winchester Addition & Alteration	3985 Valley Pike Winchester, VA 		
CMA 3951 Valley Pike Winchester, VA			
I certify that these documents were prepared or approved by me, and that I am a duly licensed engineer under the laws of the Commonwealth of Virginia, license number: 21184 ; expiration date: 12-31-2023.			
Bid Set No. Issue / Revision	2023.07.27 Date		
Drawn By: Checked By: Plot Date:	HAG MWD July 28, 2023		
Sheet Number S-103			
^{Sheet Title} MEZZ. FRAMING PLAN			
Project Number 23-044DL	File Name 23-044DL-CMA Honda Winchester-PDG S-102X_ROOF FRAMING PLAN.dwg		

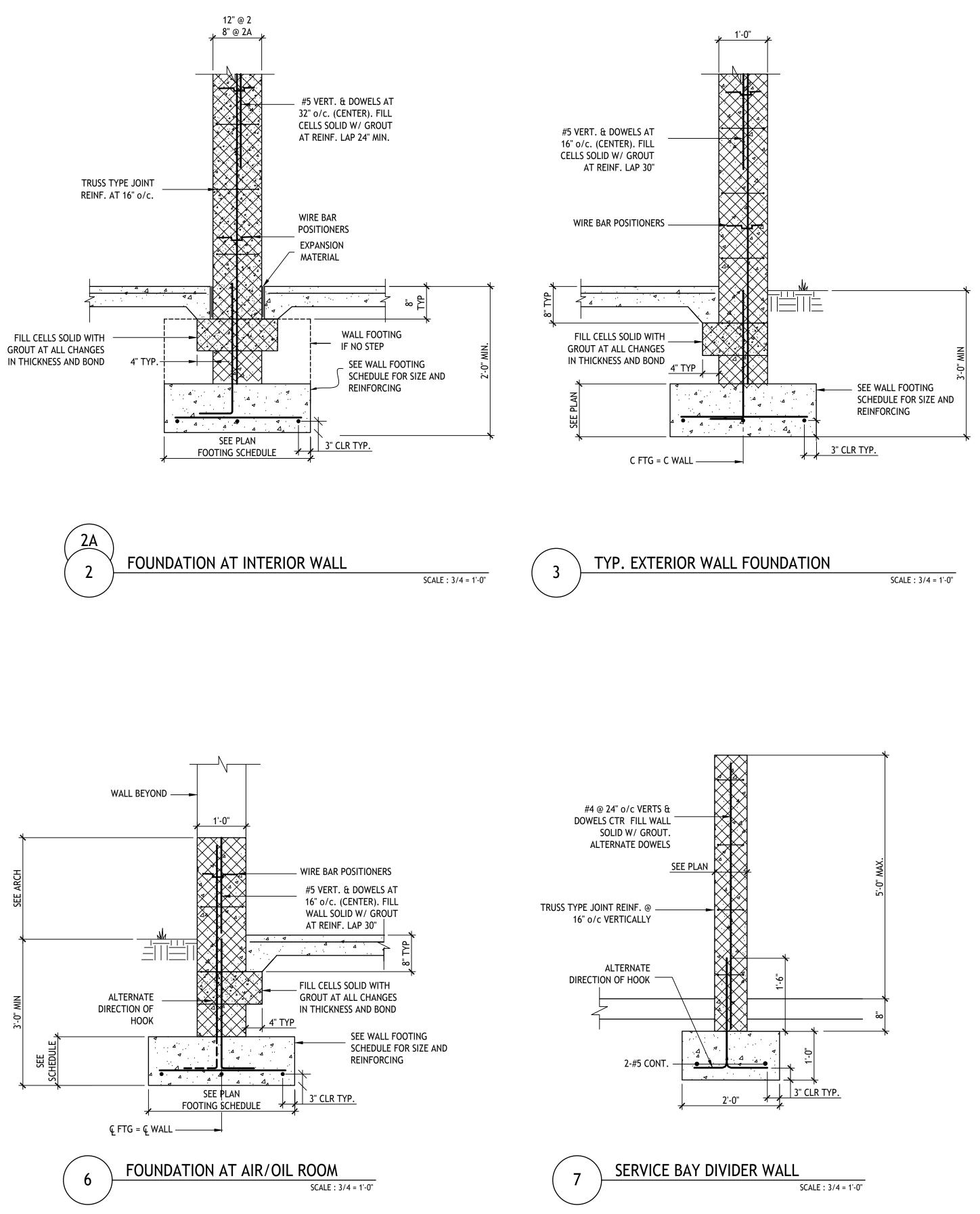


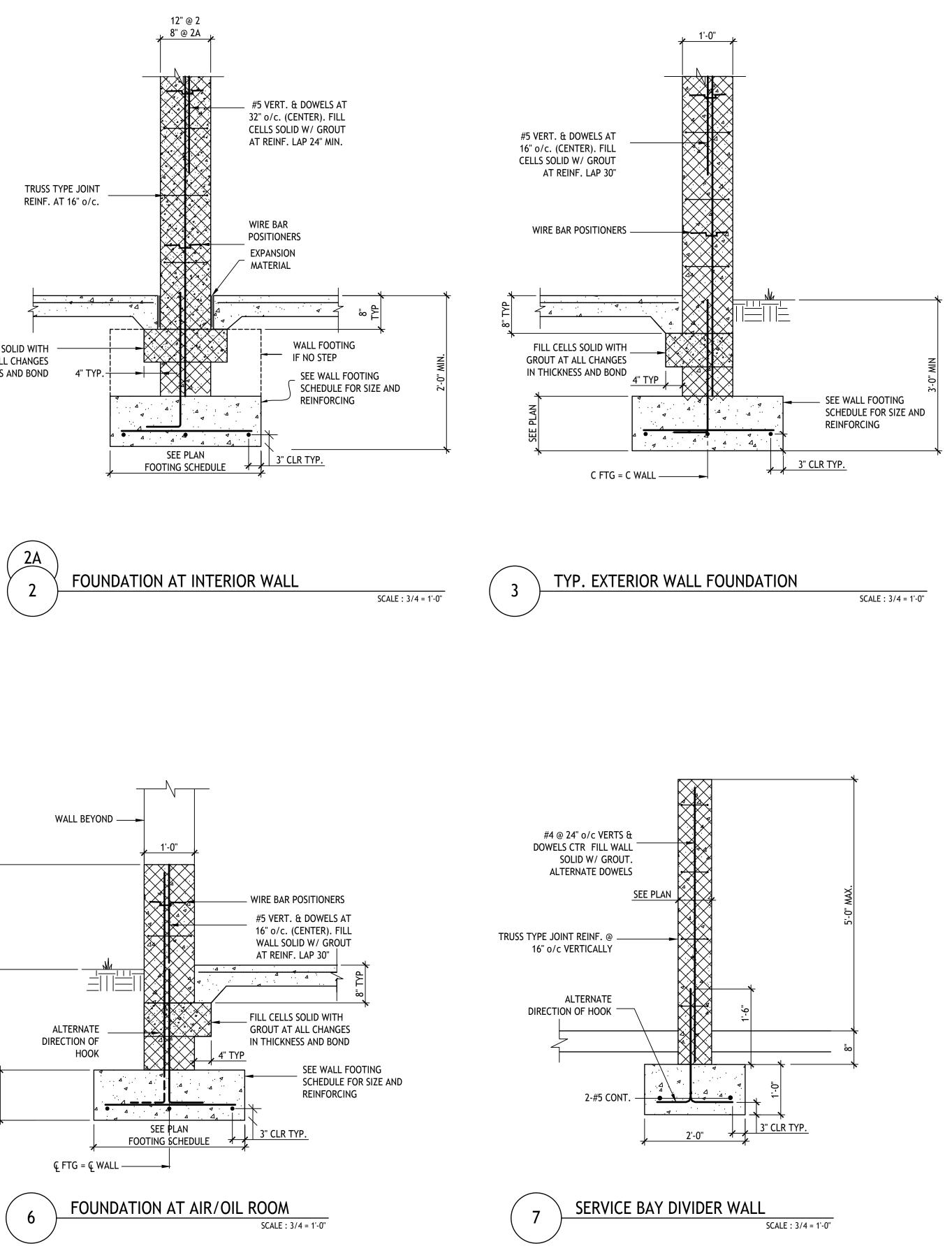


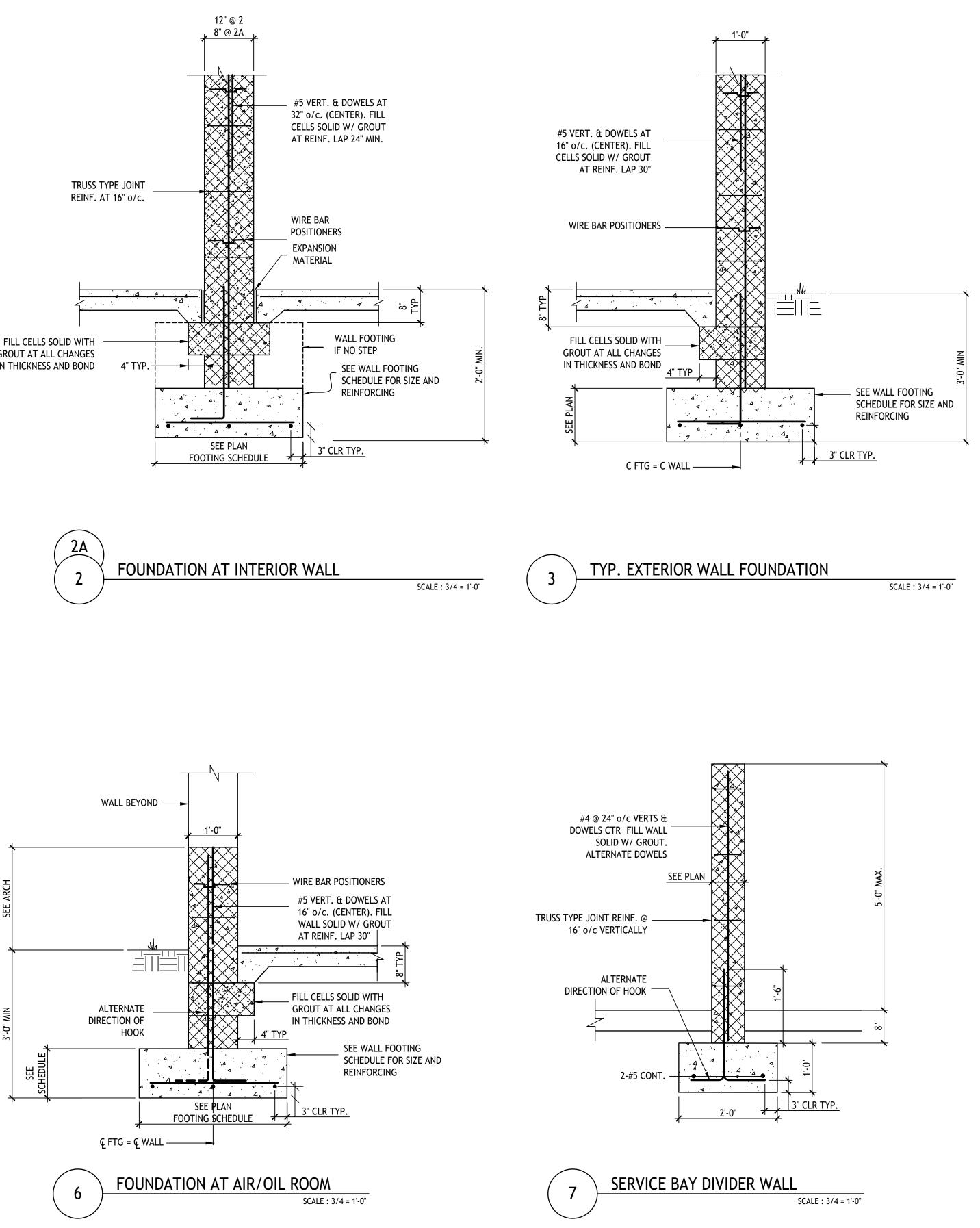


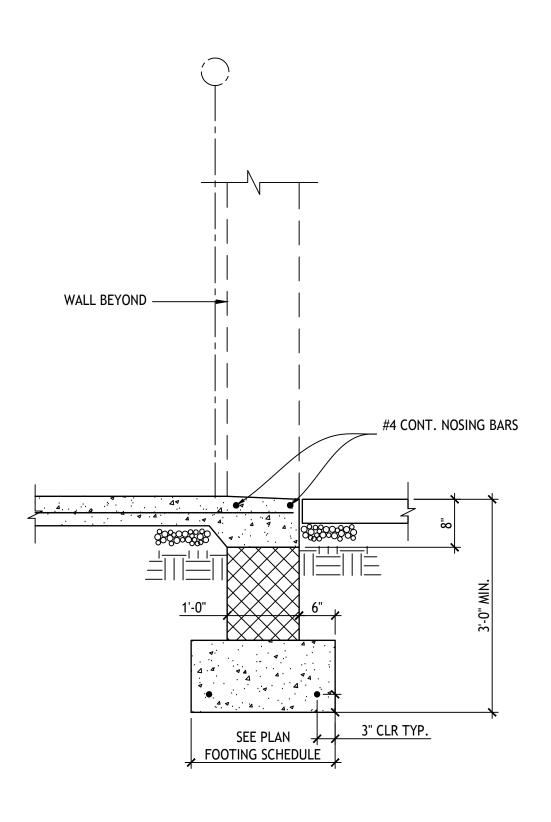
Date











TYP. EXTERIOR WALL FOUNDATION @ DOOR OPENING 4 SCALE : 3/4 = 1'-0"

